
1 **First excursion probability of dynamical systems:** 2 **A review on computational methods**

3 **Youbao Jiang^a, Xuyang Zhang^{a,*}, Michael Beer^{b,c}, Matthias G.R.**
4 **Faes^d, Costas Papadimitriou^e, Hao Zhou^a**

5 ^a*School of Civil Engineering, Changsha University of Science and Technology, Changsha 410114, China*

6 ^b*Institute for Risk and Reliability, Leibniz University Hannover, Callinstr. 34, Hannover 30167, Germany*

7 ^c*Institute for Risk and Uncertainty, University of Liverpool, Liverpool L69 7ZF, United Kingdom*

8 ^d*Chair for Reliability Engineering, TU Dortmund University, Leonhard-Euler-Straße 5, Dortmund 44227,*
9 *Germany*

10 ^e*Department of Mechanical Engineering, University of Thessaly, Volos 38334, Greece*

11 **Highlights**

- 12 • We provide a synthesis of the literature on the first excursion
13 probability of dynamical systems and their recent developments.
- 14 • We discussed the efficacy of existing methods in addressing various
15 challenges associated with first excursion problems in complex
16 dynamic systems.
- 17 • We present a synthesis of the most recent methods proposed in the
18 field of reliability and evaluate their potential for addressing complex
19 first excursion problems.

20 **Abstract**

21 The theory of dynamic reliability, predicated on the first excursion
22 failure criterion, holds significant importance in the domains of seismic
23 and wind resistance of structures, as well as in the assessment of the
24 reliability of machinery and airplanes. This theoretical framework offers a
25 mathematical description of failure probabilities, which serve as critical
26 indicators for the safety evaluations of dynamic systems. However,
27 dynamical systems such as large structures, machines or airplanes are
28 composed of numerous members and nodes that may be influenced by
29 uncertainties related to loads, geometric imperfections, and material
30 properties. The inherent high-dimensional randomness and pronounced
31 nonlinear coupling effects contribute to the complexity and implicit
32 nature of the system failure modes in these systems. Consequently, the
33 computation of the first excursion probability for complex dynamical

34 systems presents a formidable challenge that necessitates comprehensive
35 investigation. To summarize the current methodologies, this paper
36 delineates a state-of-the-art review of dynamic reliability theory, with a
37 particular emphasis on its potential to address the first excursion
38 probability in dynamical systems.

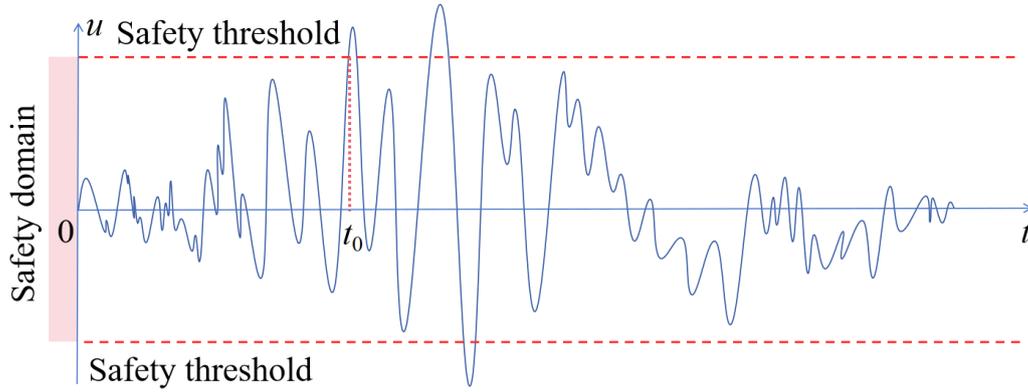
39 **Keywords:** Reliability theory, First excursion probability,
40 Dynamical systems, Random excitation, Random vibrations

41 **Introduction**

42 In the processes of designing and assessing structural systems,
43 natural hazards, such as wind loads and earthquakes, can lead to the
44 partial failure or collapse of structures, resulting in catastrophic damage.
45 Similar phenomena are observed when dealing with mechanical
46 engineering systems such as vibration induced by imbalances in rotating
47 machines or turbulence loads on aircraft wings. Therefore, it is essential
48 to evaluate the reliability performance of dynamical systems under
49 stochastic loads to mitigate potential safety risks. While some existing
50 studies utilize random variables to model extreme random processes, they
51 are limited in their ability to analyze the transient reliability.
52 Consequently, the dynamic reliability analysis of system responses to
53 random excitation will become increasingly critical in the future [1].

54 Dynamic reliability refers to the probability that a system will
55 successfully perform its intended function when subjected to random
56 excitations within specified time frames and conditions. The first
57 excursion failure criterion assesses whether a system has lost its
58 performance by comparing physical quantities associated with failure to a
59 critical value or safety threshold. This criterion provides a simplified and
60 idealized model, as illustrated in Fig. 1. By determining whether any of
61 the random outputs $u(t)$ exceed a safety threshold level b within a
62 specified time duration T , the first excursion probability P_f can be
63 expressed as:

$$P_f = p(\exists t \in [0, T]: |u(t)| > b) \quad (1)$$



t_0 denoted the first excursion time

Fig. 1 The First excursion problem of random processes.

During the preliminary phases of investigating the first excursion probability, the emphasis was placed on examining the random process and its correlation with thresholds. In 1945, Rice formulated a set of infinite series expansions and complex multiple integral equations to address the issue of exceeding fixed boundaries in random processes. This seminal work established a foundational framework for dynamic reliability analysis rooted in the theory of random processes [2]. Subsequently, Middleton validated Rice's formula for calculating the first excursion probability and further investigated the challenges posed by random processes exceeding safety thresholds [3]. Their contributions have significantly advanced the theoretical research pertaining to the first excursion problem. Consequently, the Poisson process method, predicated on the cross assumption, has been extensively utilized in addressing the first excursion problem[4].

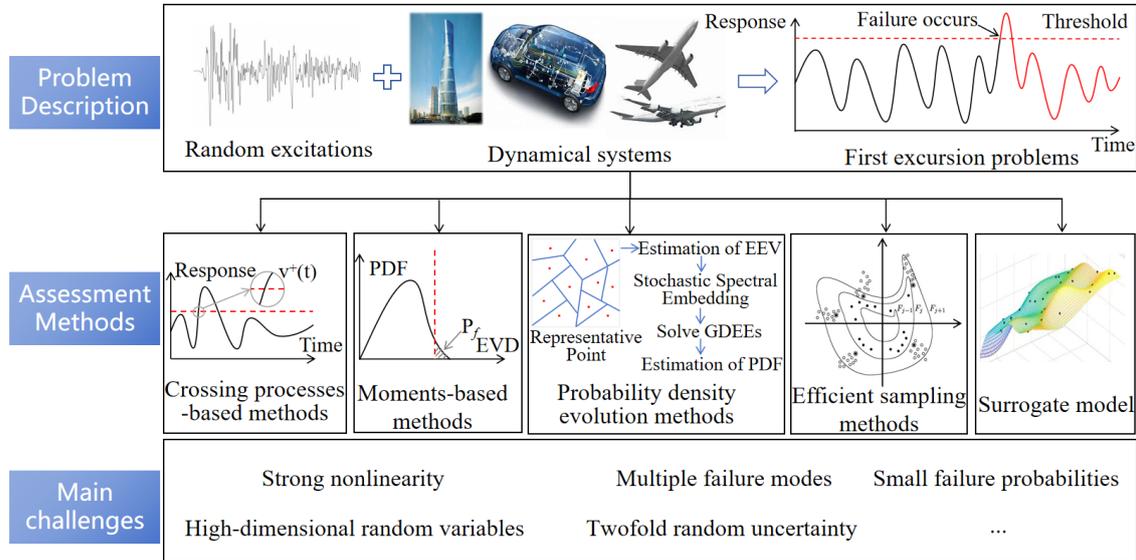
Research concerning the first excursion probability of nonlinear systems commenced in the 1970s, predominantly focusing on systems characterized by small damping, weak nonlinearity, or bilinear restoring forces. The stochastic response of dynamic systems can be solved by solving the Fokker-Planck-Kolmogorov (FPK) equation and its variations, or using equivalent linearization methods, which are still widely used in structural first excursion probability analysis[5].

87 Subsequent investigations have demonstrated that the first excursion
88 probability under specified thresholds can be equivalently derived
89 through the relevant extreme value distribution [6]. This facilitates the
90 transformation of the time-dependent dynamic reliability problem into a
91 corresponding time-invariant problem. Over the past two decades,
92 probability density evolution methods have also made significant
93 progress, providing a unified framework for dynamic and static reliability
94 analysis [7].

95 In addition to semi-analytical methods, numerical simulation
96 techniques have been employed to determine the first excursion
97 probability. Due to the high computational demands of Monte Carlo
98 simulation for extremely small probability analysis [8], several efficient
99 sampling methods [9] and surrogate model methods[10] have been
100 proposed, yielding effective solutions for the first excursion problem of
101 complex dynamical systems.

102 When conducting the first excursion probability analysis in practical
103 engineering systems, numerous challenges persist. These challenges
104 include the small failure probability problem in high-dimensional
105 nonlinear systems, the twofold random uncertainty challenge associated
106 with uncertainty of structural physical-geometrical parameters and the
107 randomness of external excitation, as well as the dynamic reliability
108 concerns associated with complex systems subjected to multiple failure
109 modes. To address the potential challenges that may arise in the first
110 excursion probability analysis of dynamic systems, this paper presents the
111 development of the first excursion probability calculation method and its
112 subsequent application in resolving these challenges. In this paper,
113 existing methods are categorized into two main categories:
114 semi-analytical methods and numerical simulation methods. For each
115 category, an overview and summary of each method are presented, along
116 with a discussion of the latest advancements. Furthermore, this paper
117 addresses the existing gaps in current methodologies and suggests

118 potential avenues for future research. The organization of the paper is
 119 illustrated in Fig. 2.



120
 121 **Fig. 2** The organization of the paper.

122 2 Semi-analytical Methods

123 2.1 Crossing processes-based methods

124 Assuming that the events of structural response exceeding the safety
 125 threshold are independent, it can be hypothesized that the crossing times
 126 follow a Poisson distribution. Based on this assumption, Coleman [11]
 127 proposed the well-known Poisson process-based method and derived
 128 analytical solutions for the first excursion probabilities. Within a given
 129 duration T , the probability of $u(t)$ crossing the threshold b in i times can
 130 be expressed as:

$$131 \quad p\{n(T) = i\} = \frac{1}{i!} \left[\int_0^T v_b(\tau) d\tau \right]^i \exp\left[- \int_0^T v_b(\tau) d\tau \right] \quad (2)$$

132 where $v_b(\tau)$ represents the crossing rate of the response $u(t)$ surpassing
 the threshold b , which expressed as:

$$133 \quad v_b = \frac{1}{2\pi} \frac{\sigma_u}{\sigma_u} \exp\left(-\frac{b^2}{2\sigma_u^2}\right) \quad (3)$$

134 where σ_u represents the standard deviation of displacement and σ_i
 135 represents the standard deviation of velocity.

For events characterized by high safety thresholds, the assumption of

136 a Poisson process is often deemed acceptable due to the rarity of
137 exceeding these thresholds. However, when the structural response is akin
138 to a narrowband process defined by slowly varying sine waves, the
139 occurrence of a crossing event in one cycle significantly increases the
140 likelihood of another crossing event in the subsequent cycle. This
141 behavior indicates that crossing events tend to exhibit clustering, thereby
142 rendering the assumption of a Poisson process inappropriate [12]. To
143 enhance the precision of first excursion probability evaluations, Corotis et
144 al. [13] advocate for the consideration of crossing events as a Markov
145 process, wherein the probability of the next crossing event is contingent
146 solely upon the current event. Yang and Shinozuka [14][15] employed the
147 point process method alongside the maximum entropy principle to
148 investigate the first excursion problem in normally distributed
149 narrowband responses. The main formula is as follows:

$$p(T) = 1 - [2(\lambda_2 / \pi)^{1/2} \exp(-\lambda_1^2) / \operatorname{erfc}(\lambda_1)] \quad (4)$$

150 where $p(T)$ represents the first excursion probability in $[0, T]$. In Eq.
151 (4), $\operatorname{erfc}(\lambda_1)$ is the complementary error function, and λ_1 and λ_2 can be
152 obtained from the following equations:

$$\mu_2 / \mu_1^2 = (\gamma - \lambda_1)^{-2} / 2 - \lambda_1 / (\gamma - \lambda_1) \quad (5)$$

$$\lambda_2 \mu_2 = 0.5 + \lambda_1^2 - \gamma \lambda_1 \quad (6)$$

153 μ_1 and μ_2 represents the mean value and the second moment of total
154 number of excursions in $[0, T]$, and

$$\gamma = \exp(-\lambda_1^2) / [\sqrt{\pi} \operatorname{erfc}(\lambda_1)] \quad (7)$$

155 Their findings suggest that the point process approach produces
156 several useful approximations for the first excursion probability,
157 particularly those based on the concepts of the Markov process, the
158 clump-size and the non-approaching random points. Subsequent research
159 has introduced various models for crossing events, including the classical
160 Vanmarcke approximation model [12][16], and the stochastic process

161 envelope model [17][18],among others.

162 Based on the assumption of a Poisson process, a critical step in
163 determining failure probability is the analysis of the out-crossing rate.
164 Consequently, the calculation of the out-crossing rate between system
165 responses and safety limits in complex scenarios has emerged as a
166 significant research objective. For instance, the PHI2 method [19]
167 expresses the out-crossing rate through a classical two-component
168 parallel system reliability model, which transforms time-variant reliability
169 into time-invariant reliability at successive time instants. Naess [20]
170 employed the random process spectral decomposition technique to
171 develop a differential calculation method for the out-crossing rate of
172 Gaussian process exceedance within the framework of the second-order
173 Volterra series. He [21] employed Simpson's numerical integration rule to
174 approximate the first three spectral moments of the linear system
175 responses subjected to non-stationary random excitation. Li et al. [22]
176 derived closed-form analytic solutions for calculating the average
177 out-crossing rate and first excursion probability of a non-stationary
178 log-normal process.Cai et al.[23] proposed a new analytical formula to
179 determine the mean outcrossing rate of nonstationary non-Gaussian
180 performance functions. These methodologies provide a quantitative
181 framework for calculating the average out-crossing rate of the system in
182 the context of complex excitations, thereby facilitating the evaluation of
183 the first excursion probability of dynamic systems across various
184 complex scenarios.

185 However, when addressing first excursion problems associated with
186 non-Gaussian excitations or nonlinear systems, it is inappropriate to
187 model the system response directly using Poisson processes.
188 Consequently, approximate methods must be employed for
189 calculations[24]. This limitation constrains the applicability of crossing
190 process-based methods in the first excursion probability analysis of
191 complex dynamic systems.

192 **2.1.1 Latest developments of crossing processes-based methods**

193 Recent research has increasingly focused on the clustering of
194 crossing events in scenarios characterized by stochastic processes
195 exhibiting high autocorrelation and low thresholds. Yi et al.
196 [25]developed a Poisson Branching Process model to effectively capture
197 the statistical dependencies among crossings, thereby facilitating a more
198 precise estimation of the first excursion probability for stationary
199 Gaussian processes. Similarly, Yang et al. [26]introduced the
200 equiprobable joint Gaussian method to mitigate the issue of redundant
201 counting of identical crossing events. They employed the conditional
202 up-crossing rate to compute the first excursion probability by utilizing
203 collectively exhaustive events and the law of total probability, thus
204 addressing the dynamic reliability challenges associated with
205 high-dimensional nonlinear systems.

206 **2.1.2 Summary of crossing processes-based methods**

207 The crossing processes-based methods represent a preliminary
208 approach to addressing the first excursion problem, providing an
209 analytical formula for calculating the first excursion probability. Although
210 these methods can produce a failure probability that approximates the true
211 value, they rely on particular assumptions regarding the crossing event.
212 Consequently, while these methods constitute a significant contribution to
213 the first excursion problem, there remains substantial potential for further
214 development in addressing the challenges encountered in complex
215 dynamic systems.

216 **2.2 Diffusion processes-based methods**

217 The stochastic vibration equation of a nonlinear system subjected to
218 white noise or filtered white noise can be reformulated as an Itô
219 stochastic differential equation. It can be demonstrated that the solution to
220 the Itô equation constitutes a Markov process. The transition probability
221 density function, which comprehensively characterizes the statistical
222 properties of the Markov process, adheres to the

223 Fokker-Planck-Kolmogorov equation (FPK equation). Consequently, the
 224 random response of the structure can be derived by solving the FPK
 225 equation. In theory, the FPK equation method constitutes the most
 226 rigorous framework for the analysis of nonlinear random vibrations,
 227 yielding precise solutions to nonlinear problems. However, solving the
 228 FPK equation presents substantial challenges. Even in scenarios
 229 involving stationary responses, only a limited number of specific systems
 230 permit the determination of exact solutions.

231 By solving the partial differential equation governing the system's
 232 response, such as the Kolmogorov backward equation derived from
 233 diffusion theory, it becomes feasible to evaluate the first excursion
 234 probability[27]. Assuming that the system state X_t evolves according to
 235 the stochastic differential equation, the Kolmogorov backward equation
 236 can be expressed as:

$$\frac{\partial p(x,t)}{\partial t} = \mu(x,t) \frac{\partial p(x,t)}{\partial x} + \frac{1}{2} \sigma^2(x,t) \frac{\partial^2 p(x,t)}{\partial x^2} \quad (8)$$

237 where $p(x,t)$ represents the probability of the random process being in
 238 state x , and t represents time. In Eq. (8), $\mu(x,t) \frac{\partial p(x,t)}{\partial x}$ represents the rate
 239 of state transition and $\mu(x,t)$ is called the drift term; $\frac{1}{2} \sigma^2(x,t) \frac{\partial^2 p(x,t)}{\partial x^2}$
 240 represents the diffusion rate and $\sigma^2(x,t)$ is called the diffusion term.

241 Under specific conditions, the dynamic response of a nonlinear
 242 system subjected to non-white noise random excitation may be
 243 approximated by a Markov diffusion process. The drift and diffusion
 244 coefficients of the FPK equation associated with this approximate
 245 diffusion process can be derived from the motion equations of the given
 246 system, expressed as:

$$\dot{A} = K_1(A,t) + K_2(A,t)\eta(t) \quad (9)$$

$$K_1(A,t) = -\frac{1}{2} \varphi(A)A(t) + \frac{\pi S[\omega(A),t]}{2A(t)\omega^2(A)} \quad (10)$$

$$K_2(A,t) = \frac{\{\pi S[\omega(A),t]\}^{1/2}}{\omega(A)} \quad (11)$$

247 where the response amplitude envelope A is a slowly varying function
 248 with respect to time and can therefore be treated as a constant over one
 249 cycle of oscillation, which approximately governs the temporal evolution
 250 of the amplitude $A(t)$; $\eta(t)$ is a zero mean and delta correlated Gaussian
 251 process of intensity 1, i.e., $E[\eta(t)] = 0$; and $E[\eta(t)\eta(t+\tau)] = \delta(\tau)$, with $\delta(\tau)$
 252 being the Dirac delta function. In Eq. (10) and Eq. (11), $\varphi(A)$ and
 253 $\omega^2(A)$ represent the equivalent damping element and the natural frequency
 254 of the system, respectively, while $S[\omega(A),t]$ represents the evolutionary
 255 non-separable power spectrum of the form. The Fokker-Planck (F-P)
 256 equation, which corresponds to stochastic differential equations, is cast in
 257 the form:

$$\frac{\partial}{\partial t} p(A,t|A',t') = -\frac{\partial}{\partial A} [K_1(A,t)p(A,t|A',t')] + \frac{1}{2} \frac{\partial^2}{\partial A^2} [K_2^2(A,t)p(A,t|A',t')] \quad (12)$$

258 The coefficients of this approximate diffusion process equation can
 259 be derived through the appropriate application of random averaging to the
 260 motion equation of the given system, and the solution of the averaged
 261 FPK equation can yield an approximate response of the original
 262 system[28]. This approach is particularly effective for analyzing the
 263 stochastic response of lightly damped nonlinear SDOF systems subjected
 264 to broadband random excitation. Additionally, approximate analysis
 265 techniques based on random averaging and statistical linearization can
 266 effectively address various nonlinear and lagging systems, as well as
 267 diverse forms of random excitation, while maintaining low computational
 268 costs [29]. Building upon this foundation, Mitseas et al. [30][31]
 269 developed a stochastic incremental dynamic analysis method for
 270 calculating the first excursion probability, which is applicable to
 271 nonlinear structural systems influenced by random seismic excitation, as
 272 well as lagged fractional order structural systems under entirely
 273 non-stationary seismic excitation vectors.

274 The responses PDF of the nonlinear stochastic system excited by
 275 white noise is approximated with the exponential function of a
 276 polynomial in state variables. The exponential polynomial closure (EPC)
 277 method [32] is employed to satisfy the FPK equation in the weak sense of
 278 integration with the assumed PDF. In the context of a nonlinear system
 279 excited by a discrete Poisson pulse, the probability density function of the
 280 system can be characterized through the Kolmogorov-Feller equation. By
 281 solving the Kolmogorov-Feller backward equation, the first excursion
 282 probability can subsequently be derived [33][34]. Based on the
 283 aforementioned concept, Zhu et al. [35] investigate nonlinear systems
 284 subjected to Poisson white noise excitation by solving the truncated
 285 Kolmogorov-Feller equation. Subsequent research has further analyzed
 286 the first excursion probability of nonlinear oscillators under combinations
 287 of composite noise, utilizing advanced EPC methods [36][37].

288 2.2.1 Path Integral approach

289 On the other hand, the Path Integral (PI) approach can also estimate
 290 the response PDF and the first excursion probability of low-dimensional
 291 nonlinear systems [38]. In essence, the PI approach constitutes a discrete
 292 version of the Chapman-Kolmogorov (CK) equation, which is also
 293 associated with Markov processes. The fundamental equation of the PI
 294 approach can be expressed:

$$p_Z(z, t + \tau) = \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} p_Z(z_1, z_2, t + \tau | \bar{z}_1, \bar{z}_2, t) p_Z(\bar{z}_1, \bar{z}_2, t) d\bar{z}_1 d\bar{z}_2 \quad (13)$$

295 where for compactness $p_Z(z, t + \tau) = p_Z(z_1, z_2, t + \tau)$, and $p_Z(z_1, z_2, t + \tau | \bar{z}_1, \bar{z}_2, t)$
 296 is the so-called Conditional PDF of the response vector process $Z(t)$.
 297 $Z(t)$ is a two-dimensional Markov vector process that satisfies the CK
 298 equation, and

$$\dot{Z}(t) = f(Z, t) + 1V(t) \quad (14)$$

299 where $V(t)$ is a modulated white noise process, and $\dot{Z}(t)$ is state
 300 variables of nonlinear system, $Z(t) = [Z_1(t) Z_2(t)]^T = [X(t) \dot{X}(t)]^T$, $1 = [0 \ 1]^T$ and

$$f(Z,t) = \begin{bmatrix} Z_2 \\ -f(Z_1, Z_2, t) \end{bmatrix}^T \quad (15)$$

301 The initial PI method has been extensively utilized to assess the
302 response PDF [39][40] and first excursion probability [41][42] of
303 nonlinear systems subject to time-modulated normal white noise
304 excitation. Subsequent investigations have further explored the method
305 considering the case of non-normal excitations, such as Poisson noise
306 [43], Lévy white noise [44], and composite parameter excitation [45][46].

307 To address the issue of low computational efficiency associated with
308 PI solutions in high-dimensional MDOF systems, various numerical
309 schemes have been employed to enhance the integration calculation of the
310 CK equation [47][48][49]. Currently, several advanced PI methods have
311 been successfully implemented in the context of the first excursion
312 problem related to practical engineering structures [50][51], thereby
313 demonstrating the potential of these methods in addressing the complex
314 first excursion problem.

315 **2.2.2 Latest developments of diffusion processes-based methods**

316 Recent research has extended existing methods to transient
317 probabilistic analysis of nonlinear systems under complex excitations,
318 including multiplicative and additive modulated stochastic excitations
319 [52], combined harmonic and modulated Gaussian white noise excitations
320 [53], correlated multi-power velocity multiplicative excitations, and
321 additive excitations [54], as well as multiple external and parametric
322 excitations [55]. These investigations provide a valuable reference for the
323 first excursion probability analysis of dynamical systems under complex
324 excitation conditions. Guo et al. [56] enhanced the EPC method by
325 integrating time basis functions into the PDF approximation, thereby
326 achieving entirely non-stationary PDF solutions. This implementation
327 facilitates the acquisition of continuous PDF distributions in the time
328 domain, significantly improving computational efficiency without the
329 need for repeated calculations.

330 The PI method has garnered considerable attention in recent years.
331 For example, the GPU computing-based PI strategy enhances the
332 efficiency of probability response analysis in high-dimensional systems
333 [57][58]. The combination of Laplace integration and PI method can
334 significantly mitigate associated computational costs [59], and has been
335 successfully employed in the first excursion probability analysis of
336 nonlinear SDOF systems that incorporate fractional derivative elements
337 [60], as well as under stationary and time-modulated white noise
338 excitation [61].

339 Furthermore, the application of artificial neural networks for
340 addressing the FPK equation has garnered significant attention in recent
341 literature [62][63]. Wang et al. [64] introduced a separable Gaussian
342 neural network methodology to derive solutions for the FPK equation
343 within high-dimensional state spaces, successfully obtaining solutions for
344 the high-dimensional FPK equation associated with nonlinear dynamical
345 systems.

346 **2.2.3 Summary of diffusion processes-based methods**

347 In theory, the diffusion process-based methods is the most rigorous
348 tool in nonlinear random vibration analysis, which can determine the
349 exact solution of the first excursion probability of nonlinear dynamical
350 systems. However, solving the FPK equations for complex systems is
351 extremely difficult, and currently there is a relative lack of effective
352 solutions for high-dimensional FPK equations. Therefore, numerous
353 approximation methods grounded in diffusion process theory have been
354 extensively proposed and progressively applied to the first excursion
355 problem associated with complex dynamic systems, indicating a
356 substantial potential for further development in this area.

357 **2.3 Moments-based methods**

358 When the random response of the structure is conceptualized as a
359 sequence of discrete variables, the first excursion probability can be
360 effectively calculated using the corresponding extreme value distribution

361 (EVD) of the stochastic structural response [65]. Furthermore, by
 362 leveraging the EVD, the time-dependent dynamic reliability issue can be
 363 converted into a time-invariant equivalent. In this framework, established
 364 methodologies for time-invariant structural reliability, such as first-order
 365 reliability methods (FORM), second-order reliability methods (SORM)
 366 [66], and importance sampling [67], can be directly employed to tackle
 367 the first excursion problem.

368 The EVD of the random structural response can be derived through
 369 two methodologies: the analytical approach and the approximate
 370 approach. The analytical approach is limited to specific categories of
 371 random processes and is generally impractical for addressing the
 372 reliability challenges of complex systems. Consequently, the approximate
 373 approach to EVD has garnered significant attention within the field.
 374 Moments-based methods estimate the first-passage probability by fitting
 375 an appropriate parametric distribution model to the EVD, and the free
 376 parameters of the distribution model are obtained from the estimated
 377 moments of the EVD. The integer moments methods can be adopted to
 378 recover the EVD [6][68][69], where high-order integer moments, i.e.,
 379 skewness and kurtosis, need to be considered. For instance, Low [70]
 380 developed a new distribution called the shifted generalized lognormal
 381 distribution for fitting four moments. The PDF and CDF of a shifted
 382 generalized lognormal distribution variable Y are given by, respectively

$$f_Y(y) = \frac{\alpha}{y-b_1} \exp\left(-\frac{1}{r\varepsilon^r} \left| \ln \frac{y-b_1}{\theta} \right|^r\right), x > b_1 \quad (16)$$

$$F_Y(y) = \frac{1}{2} + \frac{1}{2} \operatorname{sgn}\left(\frac{y-b_1}{\theta} - 1\right) \rho \left(\frac{1}{r}, \frac{\left| \frac{\ln \frac{y-b_1}{\theta}}{\varepsilon} \right|^r}{r} \right), x > b_1 \quad (17)$$

383 where b_1 is the location parameter, θ is the scale parameter, $0 < \varepsilon$ and $0 < r$

384 are the shape parameters, the coefficient α is defined as
385 $\alpha = 1/[2r^{1/r} \mathcal{E}\Gamma(1+1/r)]$, here $\Gamma(\cdot)$ denotes the gamma function, $\text{sgn}(\cdot)$
386 denotes the signum function, and ρ is the lower incomplete gamma
387 function ratio, i.e., $\rho(s, z) = \int_0^z t^{s-1} e^{-t} dt / \Gamma(s)$.

388 Based on integer moment method, Zhou et al. [71] combined
389 statistical moment estimation with a displacement generalized normal
390 distribution model, developing a program based on adaptive Bayesian
391 quadrature to evaluate the first four central moments of the equivalent
392 extremum of the structural response. Zhao et al. [72] proposed a
393 fourth-moment Gaussian transformation and derived its inverse transform,
394 which maps the non-Gaussian structural response to a standard Gaussian
395 process, thereby facilitating the evaluation of the first excursion
396 probability.

397 However, it is difficult to evaluate such high-order integer moments
398 using a small sample size, due to the large variability in their estimators
399 [73]. To alleviate such difficulty, a series of methods based on non-integer
400 moments, such as fractional moments and linear moments, have been
401 developed. These methods include the maximum entropy method [74],
402 the kernel density maximum entropy method [75], and the mixture
403 distribution method [76], among others. Some of these studies provide
404 valuable references for the application of moments-based methods in
405 engineering contexts. For example, Zhang et al. [77] proposed an
406 efficient method for the dynamic reliability assessment of nonlinear
407 structures subjected to non-stationary ground motions utilizing linear
408 moments. Their method for estimating the first four linear moments of
409 extrema of structural responses is developed through the application of
410 the random function-spectral representation model and nonlinear time
411 history analysis, in which principal points of a uniform distribution are
412 incorporated for input estimation. Furthermore, Chen et al. [78] propose a
413 novel methodology for the seismic reliability evaluation of random
414 nonlinear bridges subjected to spatially varying ground motions. The

415 implementation involves the generation of spatially correlated ground
416 motions using a random function-based spectral representation method.
417 Subsequently, the uncertainty of these ground motions is characterized by
418 three fundamental random variables. The first-passage probability of
419 bridges under spatially varying stochastic ground motions is derived
420 through the equivalent EVD, which is approximated using a flexible
421 four-parameter distribution, specifically the shifted generalized lognormal
422 distribution.

423 Based on extreme value theory, the EVD estimation method employs
424 probability distribution functions to model extreme value samples, such
425 as the Gumbel distribution [79] and the generalized extreme value
426 distribution [80]. Usually, modeling the main part of the EVD probability
427 distribution is relatively straightforward, while its tail requires in-depth
428 analysis for accurate fitting. In high-dimensional nonlinear problems with
429 small failure probability characteristics, the tail of the fitted EVD
430 probability distribution may exhibit unexpected oscillations, leading to
431 inaccurate evaluations of the first excursion probability. Consequently, it
432 is imperative to pay particular attention to the extreme structural response
433 at the tail of the distribution.

434 Koo and Der Kiureghian[81] developed an algorithm for identifying
435 the sequence of design points, which determines the tail probability of the
436 nonlinear stochastic response by linearizing a limit-state surface at a
437 designated “design point.” This problem is formulated as a time-invariant
438 reliability problem defined by the limit-state function

$$g(x_0, t_n, \mathbf{v}) = x_0 - X(t_n, \mathbf{v}) \quad (18)$$

439 With the input stochastic process discretized, the random vibration
440 problem is completely defined by the vectors of random variables \mathbf{v} , the
441 former representing system properties that are random, while x_0 and t_n
442 are deterministic parameters in the limit-state function. The design point
443 in the standard normal space is the solution to the constrained
444 optimization problem

$$\mathbf{v}^*(x_0, t_n) = \arg \min \{\|\mathbf{v}\|^2 : G(x_0, t_n, \mathbf{v}) = 0\} \quad (19)$$

445 This point is defined within the space of standard normal random
 446 variables derived from discretizing the input excitation, corresponding to
 447 the most probable realization of the excitation that leads to the tail
 448 exceedance event. Through this approach, FORM and SORM
 449 approximations are implemented at the design point, and can also be used
 450 to select sampling distributions in important sampling methods. For small
 451 probability problems, the probability content of the failure domain is
 452 concentrated around the design point. Therefore, design points are
 453 essential in most methods for failure probability estimation [82].
 454 Additionally, Alibrandi and Der Kiureghian [83] developed a gradient-free
 455 method for locating the design point in nonlinear stochastic dynamic
 456 analysis.

457 Based on the definition of design point, Fujimura and Der
 458 Kiureghian [84] proposed that the tail probability of the linear system is
 459 equal to the first-order approximation of the tail probability of the
 460 nonlinear system, this property motivating the name Tail Equivalent
 461 Linearization Method (TELM). The method employs a discrete
 462 representation of the stochastic excitation and concepts from the FORM.
 463 For a specified response threshold of the nonlinear system, the equivalent
 464 linear system is defined by matching the design points of the linear and
 465 nonlinear responses in the space of the standard normal random variables
 466 obtained from the discretization of the excitation. The first-order
 467 approximation of the tail probability is then given by

$$p[x_0 \leq (t_n, \mathbf{v})] = p[G(x_0, t_n, \mathbf{v}) \leq 0] = \Phi[-\beta(x_0, t_n)] \quad (20)$$

468 where $\Phi[\cdot]$ is the standard normal cumulative probability function and

$$\beta(x_0, t_n) = \alpha(x_0, t_n) \mathbf{v}^*(x_0, t_n) \quad (21)$$

469 is the reliability index, in which

$$\alpha(x_0, t_n) = -\frac{\nabla_u G(x_0, t_n, v^*)}{\|\nabla_u G(x_0, t_n, v^*)\|} \quad (22)$$

470 is the normalized negative gradient vector of the limit-state function at
471 the design point. The latter two quantities uniquely define the hyper plane
472 $\beta(x_0, t_n) - \alpha(x_0, t_n)v = 0$, which is tangent to the limit-state surface at the
473 design point.

474 Broccardo et al. [85] provided an overview of TELM and discussed
475 its potential advantages. Compared to traditional linearization methods,
476 TELM exhibits greater accuracy in estimating the distribution of tail
477 region responses, rendering it particularly suitable for high-reliability
478 problems. Garrè and Der Kiureghian [86] extended TELM from the time
479 domain to the frequency domain and application to marine
480 structures. Additionally, TELM has been applied to nonlinear mechanical
481 systems subjected to multiple random excitations [87], inelastic
482 multisupport structures subjected to spatially varying stochastic ground
483 motion [88], and structures with degrading materials [89][90]. These
484 studies indicate that TELM can effectively solve the first excursion
485 problem of nonlinear dynamical systems under random excitation.

486 **2.3.1 Latest developments of moments-based methods**

487 At present, determining the EVD of random processes remains an
488 important yet challenging problem in engineering. With the development
489 of reliability methods, new solutions have emerged for the dynamic
490 reliability problem of complex stochastic systems. For example, Chen et
491 al. [91] developed a fractional moments-based mixture distribution
492 approach to estimate the first excursion probability of high-dimensional
493 nonlinear stochastic dynamic systems. Broccardo and Der Kiureghian [92]
494 introduced the Evolution Tail Equivalent Linearization Method, which is
495 based on the approximate algorithm of Priestley's evolution theory, to
496 analyze the transient response of the entire nonlinear stochastic
497 dynamical process. Chen and Lyu [93] developed a numerical method
498 that combines the Chapman-Kolmogorov equation with the Itô stochastic

499 differential equation to evaluate the time-varying probability density
500 function (PDF) of the maximum value of a Markov process or Markov
501 vector process. By constructing an augmented Markov vector process,
502 Lyu et al.[94] calculated the PDF of time-varying extreme value
503 processes in Poisson white noise driven dynamical systems. This method
504 is applicable to high-dimensional nonlinear stochastic dynamical systems
505 and demonstrates calculation accuracy on the order of magnitude of
506 $10^{-4}\sim 10^{-5}$ at the tail of the cumulative distribution function. Based on
507 these methods, the challenges associated with high-dimensional
508 parameters, strong nonlinearity, and small failure probability in the
509 dynamic reliability analysis of dynamical systems have been effectively
510 addressed.

511 Additionally, it is noteworthy that some of the latest studies
512 simultaneously consider the uncertainty of structural physical-geometrical
513 parameters and the randomness of external excitation. For instance, Weng
514 et al. [95] proposed a conditional extreme value distribution method to
515 estimate the structural first excursion probability, which incorporates the
516 inherent randomness of structural physical-geometrical parameters as
517 well as external excitations. To extract a small failure probability, the
518 proposed method introduces an intermediate event to represent the
519 realizations of extreme structural responses in the tail of the distribution.
520 Dang et al. [96] improved the mixture distribution method by introducing
521 the moment-generating function to characterize the EVD. This method is
522 capable of accommodating both the randomness arising from structural
523 properties and external seismic excitations, and it is applicable for
524 evaluating multiple EVDs in a single analytical run. Furthermore, it has
525 demonstrated high accuracy in assessing the EVD and small first
526 excursion probability of nonlinear structures with random parameters
527 under stochastic seismic excitations. The findings of these studies
528 indicate that structural random parameters play a significant role in
529 dynamic reliability analysis. Neglecting the randomness associated with

530 structural characteristics may substantially underestimate structural
531 failure probabilities, particularly in high-reliability contexts.

532 **2.3.2 Summary of moments-based methods**

533 The first excursion problem can be reformulated as a probabilistic
534 assessment of the EVD through the computation of high-order moments
535 of the dynamic response. The parameter distribution model-based EVD
536 approximation method has been validated as an effective approach for
537 addressing most nonlinear problems.

538 On the other hand, TELM based on design point excitation, offers
539 enhanced analytical capabilities for examining the tail of the EVD and
540 addressing the issue of small failure probability in nonlinear
541 systems. Many algorithms, i.e. surrogate modeling, line sampling, subset
542 simulation and cross entropy, are in fact all related to the design point
543 concept.

544 Recent research has successfully addressed the first excursion
545 problem associated with high-dimensional parameters, strong nonlinearity,
546 and small failure probability by advancing existing time-invariant
547 methods. Relevant algorithms have been developed that consider the
548 uncertainty of structural physical geometric parameters and the
549 randomness of external excitations. While many challenges in complex
550 dynamical systems have been effectively tackled, there remains a relative
551 paucity of research concerning the multiple failure modes of complex
552 dynamical systems based on time-invariant methods.

553 **2.4 Probability density evolution methods**

554 In 2003, Li and Chen [7] developed the Generalized Density
555 Evolution Equation (GDEE) by applying the principle of probability
556 conservation to characterize random events and to decouple the control
557 equations of structural systems. They introduced a Probability Density
558 Evolution Method (PDEM) that is grounded in the fundamental concepts
559 of physical stochastic systems and the principle of probability
560 conservation, thereby providing a unified theoretical framework. This

561 method elucidates the probability density and evolutionary characteristics
562 of stochastic dynamical systems.

563 Denote the PDF of the random process $u(t)$ as $p(x,t)$. Consider the
564 probability transition process in a fixed domain D with the boundaries ∂D
565 in the state space. The total increment of probability in D over time
566 interval $[t, t+\Delta t]$ yields

$$\Delta P_d = \int_D p(x, t + \Delta t) dx - \int_D p(x, t) dx = \int_D \frac{\partial p(x, t)}{\partial t} dx \Delta t + o(\Delta t) \quad (23)$$

567 where $o(\Delta t)$ is an infinitesimal of higher order. By deriving the
568 corresponding generalized probability density evolution equation and
569 solving the GDEE, one can obtain the probability information regarding
570 the system's response. Consequently, the probability density evolution
571 method facilitates the determination of the probability density function
572 for the structural response under random excitation.

573 Within the framework of probability density evolution theory, two
574 primary categories of methods are employed for dynamic reliability
575 evaluation: the absorption boundary process method and the extreme
576 value distribution method. The absorption boundary process method
577 incorporates absorption boundary processes associated with the first
578 excursion failure criterion into the generalized density evolution equation.
579 By solving the probability density function of the structure within the
580 safety domain, this method calculates the dynamic reliability of the
581 structure through integration [97]. Conversely, the extreme value
582 distribution method converts the dynamic reliability of the structure into a
583 one-dimensional integration problem involving the probability density
584 function of the extreme value of the dynamic response. This
585 transformation is achieved by constructing an equivalent extreme value
586 (EEV) event of the structural dynamic response over a specified time
587 period and subsequently solving it [98].

588 In comparison to sampling methods, the probability density
589 evolution method exhibits superior efficiency and accuracy in addressing
590 the dynamic reliability of simple systems [8]. Most practical engineering

591 problems are characterized by high-dimensional parameters and
592 pervasive nonlinearity. Although the analysis of the GDEE within
593 probability density evolution methods poses significant challenges,
594 numerical methods can be employed to facilitate its solution [99].
595 Consequently, the implementation of the probability density evolution
596 method in practical scenarios generally encompasses two critical steps:
597 the partitioning of the probability space and the numerical solution of the
598 GDEE.

599 In the domain of probability space segmentation and the selection of
600 representative points, Li et al. have developed a range of methodologies.
601 These include the mapping dimensionality reduction method [100], the
602 sphere of contact method [101], the number theoretical method [102], the
603 new discrepancy method [103], the partition of the probability-assigned
604 space method [104], and the GF-discrepancy method [105].

605 The Generalized Density Evolution Equation (GDEE) for stochastic
606 dynamical systems bears resemblance to the conservation equation in
607 fluid mechanics. Consequently, it can be characterized as a first-order
608 hyperbolic partial differential equation with time-varying coefficients,
609 which presents a significant challenge for direct numerical solution
610 techniques. The initial approach to solving the GDEE utilized the finite
611 difference method, specifically employing the Lax-Wendroff scheme.
612 However, this method fails to ensure the preservation of non-negativity in
613 the probability density function. This inadequacy necessitated the
614 integration of a flux limiter to amend the Lax-Wendroff scheme and
615 ascertain that the differential scheme retains the Total Variation
616 Diminishing (TVD) property. Shi et al. [106] developed a non-uniform
617 time step TVD scheme founded on non-uniform grid partitioning
618 technology, thereby introducing an improved probability density
619 evolution equation TVD scheme. The spatial and temporal discretization
620 of the differential method is also constrained by the
621 Courant-Friedrichs-Lewy condition, a challenge addressed by the

622 Petrov-Galerkin finite element approach to the GDEE proposed by
623 Papadopoulos and Kaloggeris [107]. Furthermore, the δ function
624 sequence solution method introduced by Fan and Li offers an innovative
625 numerical solution for the generalized density evolution equation
626 [108][109].

627 To mitigate potential issues related to grid sensitivity that may arise
628 during the GDEE solution, Tao and Li [110] proposed a novel
629 difference-wavelet method. This method enhances the finite difference
630 results through the application of nonlinear wavelet density estimation.
631 By determining the optimal scale at each time step, this approach yields
632 more accurate probability density outcomes. Additionally, they developed
633 an integrated evolution method for solving the probability density
634 evolution equation, which incorporates the standard deviation information
635 from each probability subdomain into the probability density evolution
636 equation. Subsequently, they employed a point selection technique based
637 on the Gauss-Frobenius difference, alongside a finite difference format
638 characterized by diminishing total variation, to solve the updated GDEE
639 [111].

640 Li et al. [112] conducted a study on the application of the probability
641 density evolution method in the dynamic reliability analysis of nonlinear
642 systems. Their findings indicate that the response probability density of
643 random nonlinear structures exhibits typical evolutionary characteristics.
644 However, as time progresses, the evolution process becomes increasingly
645 complex, resulting in significant random fluctuations in the response of
646 the nonlinear system. Although certain enhancement algorithms can
647 address general nonlinear problems [113], challenges remain when
648 confronting high-dimensional complex nonlinear systems.

649 In order to better handle the first excursion problem of complex
650 dynamical systems, Chen and Yang [114] developed the Direct
651 Probability Integration Method (DPIM) to characterize the propagation of
652 randomness. This method formulates a probability density integral

653 equation based on the principle of probability conservation for stochastic
654 events. By utilizing techniques for partitioning probability space and
655 applying smoothing methods with Dirac's δ function, DPIM effectively
656 addresses the integral equation to derive the probability density function
657 of the system's random response. Through the partitioning of the input
658 probability space and the application of the smoothing technique, the
659 formulations of DPIM are expressed as follows:

$$p(x,t) \square \sum_{q=1}^N \left\{ \delta[x - g(\theta_q, t)] \int_{\Omega_{\theta_q}} p_{\Theta}(\theta) d\theta \right\} \square \sum_{q=1}^N \left\{ \frac{1}{\sqrt{2\pi\nu}} e^{-[x-g(\theta_q, t)]^2/2\nu^2} P_q \right\} \quad (24)$$

660 where θ_q indicates the q -th representative point in probability space;
661 Ω_{θ_q} denotes the representative region occupied by the q -th representative
662 point; N represents the total number of representative points; ν is the
663 smoothing parameter, specifically the standard deviation of Gaussian
664 distribution; P_q means assigned probability of the q -th representative
665 point.

666 The generalized density evolution equation and the probability
667 density integral equation of dynamical systems represent differential and
668 integral formulations of the principle of conservation of probability.
669 These equations exhibit an equivalent relationship and are capable of
670 effectively addressing composite random vibration problems that involve
671 both random parameters and random excitations. Furthermore, the
672 numerical solution of the probability density integral equation does not
673 require adherence to the Courant-Friedrichs-Lewy conditions pertaining
674 to time and spatial discretization, which results in significant advantages
675 in computational accuracy and efficiency within dynamic systems [115].
676 Additionally, the DPIM is applicable to both linear and nonlinear static
677 mechanics and dynamics problems [116][117], and it can also be
678 successfully utilized for reliability assessment in practical engineering
679 applications [118][119].

680 **2.4.1 Latest developments of probability density evolution methods**

681 In recent research, Wang and Li [120] introduced a novel enrichment

682 strategy utilizing the Regenerated Kernel Particle Method (RKPM). This
683 approach necessitates reduced computational effort to obtain the
684 time-dependent probability distribution of the response of interest. By
685 employing RKPM as a surrogate model to generate more representative
686 sampling points, it leads to more precise probability density solutions
687 [121]. Das et al. [122] integrated this method with stochastic spectral
688 embedding, utilizing a limited number of representative points to estimate
689 the probability of failure. Xu [123] presented a filtering strategy grounded
690 in probability density evolution theory and Bayesian principles [124].
691 Through these methods, the computational efficiency and accuracy of
692 probability density evolution methods have been significantly enhanced.
693 Furthermore, it is noteworthy that Feng et al. [125] proposed an enhanced
694 PDEM framework that accounts for multiple failure modes and limit
695 states. Utilizing this methodology, they analyzed the dynamic reliability
696 of reinforced concrete frames under seismic excitation based on three
697 failure conditions. The findings of this research substantiate the
698 importance of considering multiple failure modes and provide a reference
699 for future reliability assessments employing PDEM for multiple limit
700 states and failure modes.

701 To establish a theoretical framework for the random response
702 analysis of multidimensional nonlinear systems and to accurately and
703 efficiently evaluate the first excursion problem of the quantity of interest
704 within a designated safety domain, Chen et al. [126] developed a
705 globally-evolving-based generalized density evolution equation
706 (GE-GDEE). This method was applied to the first excursion problem of
707 various high-dimensional nonlinear stochastic dynamical systems
708 subjected to white noise excitation. The GE-GDEE method constructs a
709 boundary absorption process for the quantity of interest within a safe
710 domain, formulates a two-dimensional partial differential equation that
711 satisfies its instantaneous probability density function, and employs
712 finite-order representative deterministic dynamic analysis data from the

713 original high-dimensional system to construct the intrinsic drift
714 coefficient using numerical techniques. Subsequently, GE-GDEE is
715 solved to derive the probability distribution of the quantity of interest
716 based on this intrinsic drift coefficient [127]. With the establishment of a
717 unified theoretical framework for the probability density evolution
718 equation applicable to general continuous stochastic processes [128], this
719 method can be extended to address more complex stochastic dynamic
720 problems, such as non-stationary stochastic excitation [129], additive
721 white noise excitation [130], stochastic wave excitation [131], twofold
722 randomness systems [132], and fractional derivative systems [133].

723 **2.4.2 Summary of probability density evolution methods**

724 The innovation and advantage of the probability density evolution
725 method lie in its decoupling of probability space and physical space,
726 enabling the simultaneous consideration of parameter uncertainty and the
727 randomness of dynamic excitation. Furthermore, the dimension of the
728 generalized density evolution equation is not constrained by the
729 dimension of the original stochastic dynamical system; it is solely
730 dependent on the dimension of the physical quantity of interest. By
731 progressively solving the system control equation along with the
732 generalized density evolution equation, it is possible to derive the
733 probability density function of the stochastic structural dynamic response.
734 Building upon this foundation, advanced methods for PDEM extend this
735 theoretical framework to complex situations, such as high-dimensional
736 nonlinearity and multiple failure modes. These methodologies introduce a
737 novel approach for the probability density evolution analysis of stochastic
738 dynamical systems and have emerged as a significant focal point in the
739 advancement of dynamic reliability calculation theory in recent years.

740 Nevertheless, there remain areas for improvement within this
741 method. For instance, although the dimension of the response under
742 investigation is typically much smaller than the system degrees of
743 freedom, the numerical computational cost associated with solving the

744 GDEE will increase markedly with each additional dimension of the
 745 response. Consequently, the expense of solving the joint instantaneous
 746 probability density function with multiple response variables remains
 747 substantial.

748 In summary, the probability density evolution theory effectively
 749 addresses high-dimensional nonlinear, small failure probabilities, and
 750 twofold random uncertainties challenges. Several studies have applied
 751 dynamic reliability analyses to practical engineering scenarios within the
 752 framework of probability density theory. Therefore, this theoretical
 753 approach holds considerable promise for addressing the first excursion
 754 problem associated with dynamical systems.

755 **2.5 The main methods and contributions of Section 2**

756 The main methods and contributions of Section 2 are presented in
 757 Table 1.

758 **Table 1** Main methods and contributions of Section 2

Author(Year)	Method	Main contributions
Rice S O(1945) MiddletonD (1960)	Surpassing boundaries theory of random processes	The research foundation for the first excursion probability is established
Coleman, J J (1959)	Poisson process assumption	
Corotis et al. (1972)	Markov process assumption	An analytical formula for the first excursion probability is proposed
Yang J N, and Shinozuka M. (1972)	point process approach,	
Iourtchenko D et al. (2008)	path integration method	An accurate solution for the first excursion probability of nonlinear systems is proposed
Spanos P D et al. (2014)	stochastic average technique	
Er G K et al. (2014)	exponential-polynomial closure method	
Der Kiureghian A (2000)	Static reliability method	Basic research framework for the first excursion problem based on time-invariant methods
Low Y M(2013)	Shifted generalized lognormal distribution	Evaluating the extreme probability distribution of structural response

He J and Gong J (2016)	Maximum entropy method	based on moment estimation method
Alibrandi U, and Mosalam K M (2018)	Mixture distribution method	
Koo H et al. (2005)	Design-point excitation	The concept of design points in first excursion problem is proposed
Fujimura K and Der Kiureghian A (2006)	Tail-equivalent linearization method	An efficient computational method has been proposed to solve the problem of small failure probability in nonlinear systems
Li and Chen (2003)	Probability density evolution method	An efficient computational method has been proposed to solve the problem of high-dimensional systems with parameter uncertainty
Chen and Yang (2019)	Direct probability integration method	The proposed method is capable of addressing both linear and nonlinear reliability problems, as well as dynamic and static scenarios

759 **3 Numerical Simulation Methods**

760 **3.1 Efficient sampling methods**

761 The randomness of system dynamic response originates from the
762 uncertainty of parameters and the randomness of excitation. Consequently,
763 the first excursion probability of a dynamic system can be accurately
764 assessed utilizing appropriate numerical simulation methods. For instance,
765 Monte Carlo methods can compute failure probabilities through extensive
766 sampling statistics, and the effectiveness and precision of this technique
767 remain unaffected by the dimensionality of the vector space or the
768 complexity of the limit state function. However, the computational
769 resources required for such large-scale sampling analyses are typically
770 substantial, presenting challenges for direct application to the first
771 excursion problem in dynamical systems. To mitigate this issue, Au and
772 Beck [7] proposed that the central issue in the first excursion problem for
773 linear systems is the concept of elementary failure regions. These regions
774 represent specific moments of failure for specific output responses. Each
775 failure region has unique characteristics that can be determined using the

776 impulse response function of the system. The complexity of the first
 777 excursion probability arises from the combination of these failure regions,
 778 which include the global design point and numerous neighboring design
 779 points. Thus, they introduced the importance sampling (IS) method to
 780 identify the design points corresponding to failure regions at different
 781 time steps based on the impulse response function. The P_f may be
 782 estimated by Eq. (25).

$$P_f \approx \hat{P}_f \times \frac{1}{N_r} \sum_{r=1}^{N_r} \frac{1}{\sum_{i=1}^m \sum_{k=1}^n \prod_{F_{ik}}(Z_r)} \quad (25)$$

783 where $\{Z_r\}_r^N$ are independent identically distributed samples simulated
 784 according to the importance sampling density function. The F_{ik} denotes
 785 the elementary failure event. m and n respectively represent different
 786 output and time steps, while N_r represents the number of sampling times.

787 The IS method in the context of the first excursion problem pertains
 788 to the identification of elementary failure regions. These regions can be
 789 comprehensively characterized by local design points, which are derived
 790 from unit impulse response functions. Consequently, IS proves to be
 791 highly effective in addressing the first excursion problem of linear
 792 systems subjected to Gaussian excitation. Numerous studies have
 793 proposed advanced methodologies based on IS, thereby enhancing the
 794 computational efficiency of failure probability calculations [134][135].
 795 Additionally, some research has sought to extend IS to accommodate
 796 nonlinear [136][137][138] or twofold random uncertainty systems [139].
 797 Nevertheless, IS continues to encounter challenges when applied to
 798 high-dimensional problems [140]. A computationally efficient sequential
 799 IS algorithm [141] based on sampling a sequence of distributions that
 800 gradually approach the optimal IS estimate is proposed that is suitable for
 801 both moderate and high dimensional problems.

802 When dealing with small failure problems in high-dimensional
 803 dynamic systems, the subset simulation (SS) method[142] exhibits

804 significant accuracy and efficiency. This method constructs a series of
805 intermediate failure events and represents the first excursion probability,
806 characterized by a small probability, as a product of larger conditional
807 probabilities of these intermediate failure events. Consequently, the
808 failure probability can be expressed as:

$$P_f = p(F_u) = p\left(\bigcap_{i=1}^u F_i\right) = p(F_1) \prod_{i=1}^{u-1} p(F_{i+1}|F_i) \quad (26)$$

809 where F_i denotes the intermediate events, and u represents the number of
810 intermediate events. Unlike SS for static reliability problems, Markov
811 chains are employed to simulate conditional sample points for estimating
812 the probability of more significant conditional failures. The SS method
813 has been demonstrated to be effective in analyzing high-dimensional
814 stochastic dynamical systems characterized by low probabilities.
815 Furthermore, it can be applied to general nonlinear problems, and its
816 efficiency is particularly notable when the intermediate threshold level is
817 appropriately chosen [143].

818 Building upon the concept of SS, several advanced methodologies
819 have been proposed. Zuev et al. [144] developed a Bayesian
820 post-processing subset simulation algorithm to generate a posterior
821 probability density function for failure probability. Liu and Yao [145]
822 selected reaction limits for the failure subspace and employed the
823 modified Metropolis algorithm to generate the random variable samples
824 necessary for calculating conditional failure probability. Katafygiotis et al.
825 [146] introduced the spherical subset simulation method, which partitions
826 the failure domain into multiple subspaces that are selected judiciously.
827 Samples for specific subspaces are generated using the Markov chain
828 Monte Carlo simulation algorithm, and the failure probability is
829 computed by aggregating the probabilities associated with each subspace.
830 Additionally, Song and Lyu [147] presented a sequence importance
831 sampling method that utilizes the fundamental concept of SS, introducing
832 reasonable intermediate failure events to partition the probability space

833 into a series of subsets. Subsequently, a sequence importance sampling
834 function is incrementally constructed to estimate the failure probability.
835 Furthermore, Katafygiotis et al. [148] proposed a domain decomposition
836 method that represents the failure domain as a combination of linear
837 failure domains delineated by hyperplane boundaries. Additionally, the
838 time-domain explicit integration method conceptualizes the structural
839 response as a summation of random coefficients and the product of
840 discrete-time random excitations [149]. By employing deterministic
841 structural time-domain analysis techniques, an explicit expression of the
842 structural response can be derived. Subsequent integration can then be
843 performed to ascertain the dynamic reliability of the structural system. Xu
844 et al. [150] combined the time-domain explicit integration method with
845 subset simulation, utilizing the Metropolis-Hastings sampling method to
846 generate samples within the conditional domain. The conditional
847 probability was computed through the time-domain explicit random
848 simulation method to address high-reliability challenges.

849 The line sampling (LS) procedure constitutes a robust and effective
850 approach for addressing high-dimensional dynamic reliability challenges,
851 facilitating the estimation of reliability for both static and dynamic
852 systems [151]. Building upon this foundation, de Angelis et al. [152]
853 introduced an advanced line sampling method that significantly reduces
854 the computational costs associated with reliability analysis, thereby
855 enabling the application of LS to dynamic reliability issues related to
856 dynamical systems. Furthermore, Papaioanou and Straub [153]
857 generalized the advanced line sampling method by proposing a
858 combination line sampling estimator. This method integrates line
859 sampling estimators corresponding to various adaptively selected
860 directions, resulting in substantial enhancements in the performance of
861 the ALS technique.

862 In addition to the aforementioned methodologies, there are also
863 methods such as directional sampling [154][155][156], directional

864 IS[157][158], sequential directional IS[159], cross-entropy-based
865 IS[160][161][162], asymptotic sampling [163], and unequal-weighted
866 sampling [164] that can be used for the first excursion probability
867 analysis of dynamic systems.

868 **3.1.1 Latest developments of importance sampling**

869 Existing research has demonstrated that efficient sampling
870 methods have advanced significantly in addressing the first
871 excursion problem. Recent developments in these methodologies
872 have further tackled challenges in the reliability analysis of
873 dynamical systems. For instance, Kanjilal et al. [165][166]
874 introduced an adaptive IS technique aimed at estimating the
875 reliability of linear structures characterized by parameter
876 uncertainties and subjected to Gaussian process excitation. The
877 directional importance sampling method proposed by Misraji et al.
878 [167] can estimate small failure probabilities with high accuracy and
879 precision, making it well-suited for numerical simulation analyses of
880 large linear structural models. Behrendt et al. [168] proposed a
881 relaxed PSD function based on SS to enhance the precision and
882 efficiency of failure probability estimates for dynamical systems.
883 Yuan et al. [169] proposed an efficient importance sampling method
884 for estimating the reliability of time-variant structures subjected to
885 time-variant loads. This method incorporates a limit state function
886 that includes random variables, structural degradation parameter
887 processes, and Gaussian stochastic load processes. Additionally, they
888 developed a simulation-based line sampling method that ensures
889 high precision in estimating the time-variant failure probability
890 function, leveraging the adaptive strategy to produce satisfactory and
891 accurate estimations of the failure probability as a function of time
892 [170].

893 **3.1.2 Summary of importance sampling**

894 With the advancement of computer technology, efficient

895 sampling methods have been extensively employed in the first
896 excursion probability analysis of dynamical systems. Due to the
897 widespread applicability of these methods, newly proposed
898 technologies frequently utilize them as benchmarks for evaluating
899 efficiency and accuracy. Currently, the improved efficient sampling
900 method has addressed the challenges associated with
901 high-dimensional parameters, small failure probability, twofold
902 random uncertainty, and strong nonlinearity in the first excursion
903 problem of complex dynamic systems. Nonetheless, there remains a
904 relative paucity of research regarding first excursion probability
905 analysis under multiple failure modes.

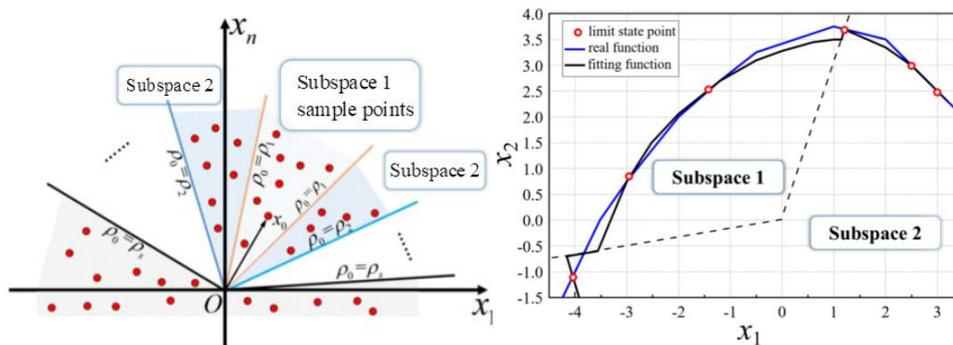
906 **3.2 Surrogate model**

907 Replacing complex numerical models with computationally efficient
908 alternatives can facilitate improved predictions of the output of interest.
909 This approach presents significant advantages in addressing intricate
910 reliability issues, as it obviates the necessity for extensive exploration of
911 structural random responses. Currently, a variety of surrogate models
912 have been employed for the analysis of first excursion probability. These
913 investigations have successfully utilized surrogate models to conduct
914 structural first excursion probability analyses, yielding accurate results.
915 At present, surrogate models such as response surfaces [171][172],
916 Hermite polynomials [173], Kriging interpolation models [174], radial
917 basis function neural networks [175], and hybrid models [176][177] have
918 been effectively applied to the first excursion probability analysis of
919 dynamical systems.

920 Notable are the surrogate techniques proposed to replace the full
921 system runs within SS method with surrogate estimates adaptively
922 constructed within each subset level. In Papadopoulos et al. [178] Neural
923 Networks (NN) were effectively trained within each subdomain generated
924 progressively at each SS level to replace full model runs in MCMC
925 chains by approximate NN predictions, providing approximate

926 cost-efficient predictions of the failure probability. Extending an adaptive
 927 kriging surrogate method that combines kriging with transitional MCMC
 928 [179] to replace full model runs if a number of conditions are fulfilled,
 929 artificial NN were adaptively trained with a fraction of the required
 930 samples per subset and implemented to generate the remaining samples
 931 within the subset of the SS method [180], substantially decreasing the
 932 total number of full model runs.

933 However, surrogate modeling techniques are difficult to accurately
 934 define the system failure boundaries when dealing with high reliability
 935 problems. Therefore, a combination of active learning techniques and
 936 surrogate models has been proposed [181]. Through active learning
 937 techniques, it is possible to train sample points close to the limit state
 938 equation adaptively, thereby minimizing the total amount of training data
 939 required. In addition, surrogate models exhibit natural advantages in
 940 addressing multi failure mode problems, as the fitting of surrogate models
 941 is only related to the shape of the limit state surface. The precise fitting
 942 algorithms for failure boundaries can identify sample points associated
 943 with different failure modes and subsequently utilize support vector
 944 machines to classify these sample points according to their respective
 945 failure modes[182][183]. As illustrated in Fig. 3, a multi-response surface
 946 model of the structural failure boundary was constructed using a zero
 947 residual fitting technique[184].



948

949

950

951

(a) Divisions of Subspaces

(b) Fitting algorithm for failure equations

Fig. 3 Efficient subspace divisions and multiple response surfaces techniques.

3.2.1 Latest developments of surrogate model

952 The latest research is still dedicated to addressing the challenges of
953 complex dynamical systems. Kim et al. [185] introduced a novel active
954 learning-based surrogate method for estimating the first excursion
955 probability of structures subjected to random wind excitation. This
956 method necessitates only a limited number of dynamic simulations to
957 yield accurate results and is anticipated to effectively address such
958 challenging and time-consuming problems encountered in practical
959 engineering contexts. Huang et al. [186] employed two types of surrogate
960 models (specifically, the back-propagation neural network model and the
961 Kriging model) in conjunction with subset simulation methods and
962 parallel computing to investigate the small failure probability problem
963 associated with stochastic structures, thereby significantly enhancing the
964 computational efficiency in scenarios involving small failure
965 probabilities.

966 **3.2.2 Summary of surrogate model**

967 The application of surrogate models to approximate the failure
968 equations of complex dynamic systems can significantly enhance the
969 computational efficiency of first excursion probability analyses. This
970 approach offers considerable advantages in addressing challenges
971 associated with nonlinearity, twofold random uncertainty, and multiple
972 failure modes, as the surrogate model's fitting relies exclusively on the
973 distribution of sample points. Furthermore, recent research has integrated
974 surrogate models with methodologies such as data dimensionality
975 reduction and adaptive sampling, effectively addressing issues related to
976 high-dimensional parameters and small failure probabilities in complex
977 dynamic systems.

978 In summary, surrogate models provide a more efficient means of
979 addressing the first excursion problem in complex systems, with their
980 accuracy being contingent upon the effectiveness of the model fitting.
981 Currently, surrogate models have been extensively utilized in static
982 reliability problems. However, there remains a relative paucity of

983 research focused specifically on the first excursion problem. As
 984 alternative modeling technologies continue to evolve, they are poised to
 985 offer viable solutions for first excursion problems in practical engineering
 986 applications.

987 **3.3 The main methods and contributions of Section3**

988 The main methods and contributions of Section 3 are presented in
 989 Table 2.

990 **Table 2** Main methods and contributions of Section 3

Author(Year)	Method	Main contributions
Au and Beck (2001)	Importance sampling	Provided an efficient sampling solution for general first excursion problems
Au and Beck (2003)	Subset simulation	Provided an efficient sampling solution for small failure probability problems
Pradlwarter et al. (2006)	Line sampling	Provided an efficient sampling solution for high-dimensional problems
Echard et al. (2011)	AK-MCS	Provided an efficient analytical tool that combines surrogate models and sampling methods
Jiang et al. (2017)	Multiple response surfaces	Provided an efficient method for multiple failure modes problems

991 **4 Potential research directions**

992 Complex dynamical systems are characterized by intricate factors,
 993 including high-dimensional parameters, strong nonlinearity, small failure
 994 probability, multiple failure modes, and twofold random uncertainty.
 995 These elements complicate the efficient and accurate evaluation of first
 996 excursion probability. Existing research on the first excursion problem
 997 pertaining to high-dimensional nonlinear systems has been extensively
 998 examined, with various algorithms proposed to tackle the issue of small
 999 failure probabilities, achieving commendable levels of accuracy.
 1000 Furthermore, some studies have explored multiple failure modes and
 1001 twofold random uncertainty, underscoring the significance of
 1002 incorporating parameter uncertainty and combinations of failure modes in
 1003 the analysis of dynamic reliability in engineering contexts.

1004 Nevertheless, the reliability analysis of practical engineering systems
1005 embodies a confluence of these factors. As a result, there remains a
1006 notable absence of a systematic research framework for the first
1007 excursion probability analysis of complex dynamical systems. To
1008 overcome this challenge, recent theoretical advancements in other
1009 disciplines may provide innovative perspectives and methodologies for
1010 engineering systems.

1011 **4.1 Wiener Path integral method**

1012 The Wiener path integral (WPI) occupies a significant position
1013 within the domain of theoretical physics, and its application across
1014 various fields of modern physics has demonstrated considerable efficacy.
1015 Kougioumtzoglou and Spanos [187] developed an approximate analytical
1016 technique to ascertain the non-stationary response probability density
1017 function of a nonlinear oscillator, drawing upon the principles of Wiener
1018 path integration and the concept of the most probable path. This
1019 methodology employs a variational formula in conjunction with the
1020 notion of the most probable path to derive an approximate solution for the
1021 Wiener path integral, thereby illustrating the analytical potential of the
1022 Wiener path integral in addressing problems related to stochastic
1023 dynamics.

1024 In subsequent studies, advanced methods based on WPI technology
1025 have been introduced, enabling the derivation of the precise joint
1026 response transition PDF of linear MDOF oscillators [188], the
1027 non-stationary joint response PDF for nonlinear MDOF systems
1028 [189][190], the stochastic response of high-dimensional nonlinear
1029 dynamic systems [191], and the stochastic response of systems
1030 incorporating fractional derivative elements [192][193]. Furthermore,
1031 several advanced techniques have significantly enhanced the
1032 computational accuracy of the Wiener path integration technique for the
1033 first excursion probability analysis of dynamical systems. For instance,
1034 Psaros et al. [194] augmented the computational efficiency of the WPI

1035 technique for ascertaining the random response of various dynamical
1036 systems through sparse representations and compressive sampling.
1037 Petromichelakis and Kougioumtzoglou [191] devised a Wiener path
1038 integral variational formula with free boundaries to determine the
1039 stochastic response of high-dimensional nonlinear dynamical systems in a
1040 computationally efficient manner. Additionally, Mavromatis and
1041 Kougioumtzoglou [195] introduced an extrapolation method within WPI
1042 technology to ascertain the random response of diverse nonlinear
1043 dynamical systems, markedly enhancing computational efficiency while
1044 preserving computational accuracy.

1045 **4.2 Bayesian failure probability inference framework**

1046 Recently, the Bayesian perspective-based reliability analysis method
1047 has garnered significant attention, interpreting the problem of integral
1048 estimation of failure probability as a Bayesian inference issue[196]. The
1049 Bayesian failure probability inference (BFPI) framework views the
1050 discretization error as a form of epistemic uncertainty, enabling it to be
1051 accurately modeled. Specifically, a prior Gaussian process is assumed for
1052 the performance function, and posterior statistics are subsequently
1053 derived for the performance function, failure indicator function, and
1054 failure probability conditional on observations obtained from evaluating
1055 the performance function at designated points.

1056 The Bayesian active learning reliability analysis methods, which
1057 integrate Bayesian inference with active learning techniques, have
1058 garnered significant attention in recent years. This approach initially
1059 conceptualizes the estimation of failure probability as a Bayesian
1060 inference problem and subsequently formulates the active learning of the
1061 failure probability based on the posterior statistics derived from the
1062 failure probability [197]. Building on this foundation, parallel adaptive
1063 Bayesian quadrature [198] can be employed to estimate extremely small
1064 failure probabilities and facilitate parallel distributed processing.
1065 Furthermore, various Bayesian active learning methods grounded in the

1066 BFPI framework have been utilized to compute extremely small failure
1067 probabilities. These methods include Partially Bayesian active learning
1068 cubature[199], Semi-Bayesian active learning quadrature [200], Parallel
1069 Bayesian probabilistic integration [201], Quasi-Bayesian Active Learning
1070 Cubature [202], and Weakly Bayesian Active Learning Quadrature [203].
1071 Collectively, these methodologies aim to reduce the number of
1072 performance function evaluations while producing failure probability
1073 estimates with specified accuracy, achieved through the establishment of
1074 reasonable stopping criteria and learning functions.

1075 Furthermore, the performance of other methodologies, such as
1076 Kriging models [204] and line sampling [205][206], can be substantially
1077 improved through the integration of Bayesian active learning.
1078 Consequently, the application of the BFPI framework to the first
1079 excursion probability analysis of dynamical systems may represent a
1080 pivotal area for future investigation. While existing methods grounded in
1081 Bayes' theorem have examined the imprecise probabilities associated
1082 with stochastic linear systems [207], research focused specifically on the
1083 first excursion probability of dynamical systems remains relatively
1084 underdeveloped.

1085 **4.3 Updating failure probability using test/monitoring data**

1086 The assessment of structural reliability depends on the uncertainties
1087 assigned to the structural model parameters and stochastic external loads.
1088 Probability distributions quantifying uncertainties in the model
1089 parameters are usually postulated based on engineering judgement and
1090 experience. Test data from material and system components collected at
1091 the initial design phase, as well as monitoring data collected during
1092 system operation, covering healthy and deteriorating conditions, provide
1093 valuable information to update prior uncertainties. Using the
1094 observation-consistent updated uncertainties, a data-informed reliability
1095 estimate can be obtained continuously over the lifetime of the system.

1096 Bayesian inference provides a consistent framework to incorporate

1097 into the reliability estimate the information contained in the data collected
1098 from the system and its components. Often the parameter set is separated
1099 into two sets, the first one consisting of the parameters that are inferred
1100 from the data, with the rest of the non-updatable parameters to be
1101 included in the second set. Such separation allows the updated failure
1102 probability integral to be conveniently re-written as a product of the
1103 conditional failure probability given the value of the first set of
1104 parameters and the Bayesian posterior distribution of the first set of
1105 parameters given the data, reducing significantly the dimension of the
1106 multidimensional integral and allowing for computational efficient
1107 algorithms to be employed.

1108 The subject of Bayesian reliability updating using test data was
1109 introduced by Papadimitriou et al. [208] and applied for the case of
1110 sufficiently large number of data, allowing for Laplace asymptotic
1111 approximation of the probability of failure integral based on asymptotic
1112 analysis of the posterior PDF given the test/monitoring data. Ni and Chen
1113 [209] combined first-order reliability methods with Bayesian mixture
1114 model to assess bridge reliability using strain measurements. Monte Carlo
1115 simulation methods have been proposed to provide an accurate estimate
1116 of updated failure probability integral by sampling the posterior
1117 distribution of the model parameters. Beck and Au [210] used an adaptive
1118 Metropolis-Hasting algorithm to generate samples of the posterior PDF
1119 and then used these samples to compute the reliability integral. Starting
1120 from samples from the posterior distribution that were generated from the
1121 transitional Markov Chain Monte Carlo method [211], Jensen et al. [212]
1122 and Hatzidoukas et al. [213] used the subset simulation method to
1123 estimate the updated failure probability. To update the reliability of linear
1124 systems under stochastic Gaussian loading and output data, Ching and
1125 Beck [214] employed an efficient IS technique, while Bansal and Cheung
1126 [215] combined Gibbs sampling for Bayesian model updating with subset
1127 simulation. Straub et al. [216] proposed the estimation of posterior failure

1128 probability based on the BUS framework [217]. Finally, Kanjilal et al.
1129 [218] introduced an efficient method for reliability updating using a
1130 two-stage cross entropy-based IS algorithm. The aforementioned updated
1131 reliability methods based on test data focus mostly on the case of
1132 epistemic uncertainties. Algorithmic developments on efficient MC
1133 simulation methods need to be explored further to reduce computational
1134 effort.

1135 Despite these developments, further research is needed to handle the
1136 aleatoric uncertainties arising due to environmental, manufacturing,
1137 operational, material and component variabilities as well as modeling
1138 errors. Hierarchical Bayesian frameworks [219][220] have been proposed
1139 to quantify and update aleatoric and epistemic uncertainties that consider
1140 the aforementioned variabilities, but very limited studies exist [221][222]
1141 to develop schemes for incorporating these uncertainties into the
1142 reliability analyses. Future developments should focus on exploring
1143 computationally efficient algorithms to handle these uncertainties within
1144 the reliability analyses, exploiting the special structure of posterior
1145 uncertainties obtained using asymptotic, variational inference and MC
1146 simulation techniques, taking into account the structural health
1147 monitoring information, as well as incorporating other types of
1148 non-updatable uncertainties such as spatial and temporal variability of
1149 external loads.

1150 **4.4 Operator norm framework**

1151 In mathematics, the operator norm serves as a quantitative measure
1152 of a linear operator between two normed vector spaces, facilitating the
1153 study of the behavior of linear operators across various contexts. Building
1154 upon the operator norm framework, Faes and
1155 Valdebenito[223][224]proposed a fully decoupled approach for a specific
1156 class of Reliability-Based Design Optimization problems, with the
1157 objective of minimizing the failure probability of linear systems subjected
1158 to stochastic excitations. Subsequent investigations have corroborated the

1159 potential advantages of the operator norm framework in reliability-based
1160 design optimization, successfully substituting the probabilistic constraints
1161 of the initial problem with deterministic constraints, thereby eliminating
1162 the need for nested loops [225][226]. Moreover, this theoretical
1163 framework can also be applied within the domain of imprecise reliability
1164 analysis, enabling the efficient determination of the first excursion
1165 probability boundary for linear [227][228]or nonlinear systems
1166 [229][230].

1167 Despite existing research indicating that the first excursion
1168 probability cannot be directly derived from the operator norm framework,
1169 the operator norm is straightforward to compute and exhibits a correlation
1170 with reliability indicators. Consequently, employing the operator norm as
1171 a surrogate for the first excursion probability in correlation analysis is
1172 highly efficient.

1173 **5 Recommendations and conclusions**

1174 This article categorizes the existing methodologies for calculating
1175 the first excursion probability and provides a comprehensive literature
1176 review of the principal approaches. Recent advancements in these
1177 methodologies indicate that challenges associated with high-dimensional
1178 parameters, nonlinearity, and small failure probabilities have been
1179 effectively addressed. Additionally, several studies have investigated the
1180 first excursion problem within the context of twofold random uncertainty
1181 or multiple failure modes, highlighting the importance of considering
1182 parameter uncertainty and multiple failure modes when analyzing the first
1183 excursion probability of dynamical systems. However, the first excursion
1184 problem related to complex dynamic systems remains lack of
1185 comprehensive researched. Therefore, the anticipated direction for future
1186 research and development is articulated as follows:

1187 (1) Currently, research regarding the first excursion probability of
1188 simple failure boundaries is relatively advanced. Nevertheless, complex
1189 dynamical systems typically consist of numerous components, and the

1190 failure mechanisms under random excitations are highly complex.
1191 Consequently, accurately identifying the primary failure modes in
1192 dynamic reliability analysis and efficiently calculating the first excursion
1193 probability of structures based on multiple failure modes remains an area
1194 that requires further investigation.

1195 (2) The failure boundaries of dynamical systems are
1196 high-dimensional and complex. Existing methodologies (e.g., probability
1197 density evolution method) frequently employ probability space
1198 partitioning techniques to identify representative sample areas. However,
1199 the proportion of sample points within significant failure boundaries is
1200 often insufficient, and some critical failure boundaries may not be
1201 included in the sampling process. As a result, both the sampling
1202 efficiency and the distribution of sample points are suboptimal, leading to
1203 time-consuming calculations of reliability with low accuracy. Utilizing
1204 limit state sampling methods or adaptive search techniques to explore
1205 structural dynamic failure boundaries may represent a significant
1206 approach to addressing the challenges associated with structural dynamic
1207 reliability under multiple failure modes.

1208 (3) Several emerging methodologies may offer solutions to existing
1209 challenges. For instance, Wiener path integration can efficiently address
1210 high-dimensional nonlinear problems. The Bayesian failure probability
1211 inference framework can accurately compute extremely small failure
1212 probabilities. Additionally, the operator norm framework effectively
1213 addresses issues related to imprecise reliability and reliability
1214 optimization problems. By further developing these novel methodologies,
1215 the current challenges associated with the first excursion problem of
1216 dynamical systems can be more effectively addressed.

1217 **Acknowledgment**

1218 This work was supported by the National Natural Science
1219 Foundation of China (Grant No.52378126); the Graduate Research

1220 Innovation Project of Changsha University of Science and Technology
1221 (Grant No.CLKYCX24018); and the German Research Foundation
1222 (Grant No. 527637016).
1223

1225 **References**

- 1226 [1] Ellingwood B, Maes M, Bartlett F M, et al. Development of methods of structural
1227 reliability. *Struct. Saf.* (2024) 102474. <https://doi.org/10.1016/j.strusafe.2024.102474>.
- 1228 [2] Rice S O. Mathematical analysis of random noise. *Bell Syst. Tech. J.* (1945) 24(1):
1229 46-156. <https://doi.org/10.1002/j.1538-7305.1945.tb00453.x>.
- 1230 [3] Middleton D. An introduction to statistical communication theory. N. Y.:
1231 McGraw-Hill, (1960).
- 1232 [4] Engelund S, Rackwitz R, Lange C. Approximations of first-passage times for
1233 differentiable processes based on higher-order threshold crossings. *Probab. Eng.
1234 Mech.* (1995) 10(1): 53-60. [https://doi.org/10.1016/0266-8920\(94\)00008-9](https://doi.org/10.1016/0266-8920(94)00008-9).
- 1235 [5] Mitseas I P, Kougioumtzoglou I A, Spanos P D, et al. Reliability assessment of
1236 nonlinear MDOF systems subject to evolutionary stochastic excitation. *Proceedings of
1237 the 7th International Conference on Computational Stochastic Mechanics (CSM 7).*
1238 Leeds, (2014).
- 1239 [6] He J, Gong J. Estimate of small first passage probabilities of nonlinear random
1240 vibration systems by using tail approximation of extreme distributions. *Struct. Saf.*
1241 (2016) 60: 28-36. <https://doi.org/10.1016/j.strusafe.2016.02.003>.
- 1242 [7] Li J, Chen J B. Probability density evolution method for dynamic response
1243 analysis of structures with uncertain parameters. *Comput. Mech.* (2004) 34: 400-409.
1244 <https://doi.org/10.1007/s00466-004-0583-8>.
- 1245 [8] Li J, Wang D. Comparison of PDEM and MCS: Accuracy and efficiency. *Probab.
1246 Eng. Mech.* (2023) 71: 103382. <https://doi.org/10.1016/j.pro bengmech.2022.103382>.
- 1247 [9] Au S K, Beck J L. First excursion probabilities for linear systems by very efficient
1248 importance sampling. *Probab. Eng. Mech.* (2001) 16(3): 193-207.
1249 [https://doi.org/10.1016/S0266-8920\(01\)00002-9](https://doi.org/10.1016/S0266-8920(01)00002-9).
- 1250 [10] Echard B, Gayton N, Lemaire M. AK-MCS: an active learning reliability method
1251 combining Kriging and Monte Carlo simulation. *Struct. Saf.* (2011) 33(2): 145-154.
1252 <https://doi.org/10.1016/j.strusafe.2011.01.002>.
- 1253 [11] Coleman J J. Reliability of aircraft structures in resisting chance failure. *Oper.
1254 Res.* (1959) 7(5): 639-645. <https://doi.org/10.1287/opre.7.5.639>.
- 1255 [12] Vanmarcke E H. First passage and other failure criteria in narrow-band random
1256 vibration: a discrete state approach. *Mass. Inst. Technol.* (1970).
- 1257 [13] Corotis R B, Vanmarcke E H, Cornell A C. First passage of nonstationary
1258 random processes. *J. Eng. Mech. Div.* (1972) 98(2): 401-414.
1259 <https://doi.org/10.1061/JMCEA3.0001591>.
- 1260 [14] Yang J N, Shinozuka M. On the first excursion probability in stationary
1261 narrow-band random vibration. *J. Appl. Mech.* (1971) 38(4): 1017-1022.
1262 <https://doi.org/10.1115/1.3408904>.
- 1263 [15] Yang J N, Shinozuka M. On the first-excursion probability in stationary
1264 narrow-band random vibration, II. *J. Appl. Mech.* (1972) 39(3): 733-738.
1265 <https://doi.org/10.1115/1.3422781>.
- 1266 [16] Ghazizadeh S, Barbato M, Tubaldi E. New analytical solution of the first-passage
1267 reliability problem for linear oscillators. *J. Eng. Mech.* (2012) 138(6): 695-706.
1268 [https://doi.org/10.1061/\(ASCE\)EM.1943-7889.0000365](https://doi.org/10.1061/(ASCE)EM.1943-7889.0000365).
- 1269 [17] Lin Y K. First-excursion failure of randomly excited structures. *AIAA J.* (1970)
1270 8(4): 720-725. <https://doi.org/10.2514/3.5747>.
- 1271 [18] Lin Y K. First-excursion failure of randomly excited structures, II. *AIAA J.* (1970)
1272 8(10): 1888-1890. <https://doi.org/10.2514/3.6013>.
- 1273 [19] Andrieu-Renaud C, Sudret B, Lemaire M. The PHI2 method: a way to compute
1274 time-variant reliability. *Reliab. Eng. Syst. Saf.* (2004) 84(1): 75-86.
1275 <https://doi.org/10.1016/j.res.2003.10.005>.
- 1276 [20] Naess A. Crossing rate statistics of quadratic transformations of Gaussian
1277 processes. *Probab. Eng. Mech.* (2001) 16(3): 209-217.
1278 [https://doi.org/10.1016/S0266-8920\(01\)00003-0](https://doi.org/10.1016/S0266-8920(01)00003-0).

1279 [21]He J. Numerical calculation for first excursion probabilities of linear systems.
1280 Probab. Eng. Mech. (2009) 24(3): 418-425.
1281 <https://doi.org/10.1016/j.probengmech.2008.12.003>.
1282 [22]Li C Q, Firouzi A, Yang W. Closed-form solution to first passage probability for
1283 nonstationary lognormal processes. J. Eng. Mech. (2016) 142(12): 04016103.
1284 [https://doi.org/10.1061/\(ASCE\)EM.1943-7889.0001160](https://doi.org/10.1061/(ASCE)EM.1943-7889.0001160).
1285 [23] Cai C H, Lu Z H, Leng Y, et al. Time-dependent structural reliability assessment
1286 for nonstationary non-Gaussian performance functions. J. Eng. Mech. (2021) 147(2):
1287 04020145. [https://doi.org/10.1061/\(ASCE\)EM.1943-7889.0001883](https://doi.org/10.1061/(ASCE)EM.1943-7889.0001883).
1288 [24] Grigoriu M. Crossings of Non-Gaussian translation processes. J. Eng. Mech.
1289 (1984) 110(4): 610-620. [https://doi.org/10.1061/\(ASCE\)0733-9399\(1984\)110:4\(610\)](https://doi.org/10.1061/(ASCE)0733-9399(1984)110:4(610)).
1290 [25] Yi S, Song J. First-passage probability estimation by Poisson branching process
1291 model. Struct. Saf. (2021) 90: 102027. <https://doi.org/10.1016/j.strusafe.2020.102027>.
1292 [26] Yang X L, Jia M M, Lu D G. The generalized first-passage probability
1293 considering temporal correlation and its application in dynamic reliability analysis.
1294 Struct. Saf. (2024) 102547. <https://doi.org/10.1016/j.strusafe.2024.102547>.
1295 [27] Spanos P D, Kougioumtzoglou I A. Galerkin scheme based determination of
1296 first-passage probability of nonlinear system response. Struct. Infrastruct. Eng. (2014)
1297 10(10): 1285-1294. <https://doi.org/10.1080/15732479.2013.791328>.
1298 [28] Dos Santos K R M, Kougioumtzoglou I A, Spanos P D. Hilbert transform-based
1299 stochastic averaging technique for determining the survival probability of nonlinear
1300 oscillators. J. Eng. Mech. (2019) 145(10): 04019079.
1301 [https://doi.org/10.1061/\(ASCE\)EM.1943-7889.000165](https://doi.org/10.1061/(ASCE)EM.1943-7889.000165).
1302 [29] Zhao Y, Psaros A F, Petromichelakis I, et al. A quadratic Wiener path integral
1303 approximation for stochastic response determination of multi-degree-of-freedom
1304 nonlinear systems. Probab. Eng. Mech. (2022) 69: 103319.
1305 <https://doi.org/10.1016/j.probengmech.2022.103319>.
1306 [30] Mitseas I P, Beer M. First-excursion stochastic incremental dynamics
1307 methodology for hysteretic structural systems subject to seismic excitation. Comput.
1308 Struct. (2021) 242: 106359. <https://doi.org/10.1016/j.compstruc.2020.106359>.
1309 [31] Mitseas I P, Ni P, Fragkouli V C, et al. Survival probability surfaces of
1310 hysteretic fractional order structures exposed to non-stationary code-compliant
1311 stochastic seismic excitation. Eng. Struct. (2024) 318: 118755.
1312 <https://doi.org/10.1016/j.engstruct.2024.118755>.
1313 [32] Er G K. Exponential closure method for some randomly excited non-linear
1314 systems. Int. J. Nonlinear Mech. (2000) 35(1): 69-78.
1315 [https://doi.org/10.1016/S0020-7462\(98\)00088-2](https://doi.org/10.1016/S0020-7462(98)00088-2).
1316 [33] Köylüoğlu H U, Nielsen S R, Iwankiewicz R. Reliability of non-linear
1317 oscillators subject to Poisson driven impulses. J. Sound Vib. (1994) 176(1): 19-33.
1318 <https://doi.org/10.1006/jsvi.1994.1356>.
1319 [34] Denisov S I, Bystrik Y S. Exact stationary solutions of the Kolmogorov-Feller
1320 equation in a bounded domain. Commun. Nonlinear Sci. Numer. Simul. (2019) 74:
1321 248-259. <https://doi.org/10.1016/j.cnsns.2019.03.023>.
1322 [35] Zhu H T, Er G K, Iu V P, et al. Exponential-polynomial closure method for
1323 solving truncated Kolmogorov-Feller equation. Int. J. Comput. Methods (2012) 9(01):
1324 1240018. <https://doi.org/10.1142/S021987621240018X>.
1325 [36] Zhu H T, Er G K, Iu V P, et al. EPC procedure for PDF solution of non-linear
1326 oscillators excited by Poisson white noise. Int. J. Nonlinear Mech. (2009) 44(3):
1327 304-310. <https://doi.org/10.1016/j.ijnonlinmec.2008.12.003>.
1328 [37] Zhu H T, Er G K, Lu V P, et al. Probabilistic solution of nonlinear oscillators
1329 excited by combined Gaussian and Poisson white noises. J. Sound Vib. (2011)
1330 330(12): 2900-2909. <https://doi.org/10.1016/j.jsv.2011.01.005>.
1331 [38] Di Paola M, Alotta G. Path integral methods for the probabilistic analysis of
1332 nonlinear systems under a white-noise process. ASCE ASME J. Risk Uncertain. Eng.
1333 Syst. Part B Mech. Eng. (2020) 6(4): 040801. <https://doi.org/10.1115/1.4047882>.
1334 [39] Naess A, Moe V. Efficient path integration methods for nonlinear dynamic
1335 systems. Probabilistic engineering mechanics.(2000) 15(2):
1336 221-231. [https://doi.org/10.1016/S0266-8920\(99\)00031-4](https://doi.org/10.1016/S0266-8920(99)00031-4).

-
- 1337 [40]Iourtchenko D V, Mo E, Naess A. Response probability density functions of
1338 strongly non-linear systems by the path integration method. *International Journal of*
1339 *Non-Linear Mechanics*.(2006) 41(5):
1340 693-705.<https://doi.org/10.1016/j.ijnonlinmec.2006.04.002>.
- 1341 [41] Kougioumtzoglou I A, Spanos P D. Response and first-passage statistics of
1342 nonlinear oscillators via a numerical path integral approach. *J. Eng. Mech.* (2013)
1343 139(9): 1207-1217. [https://doi.org/10.1061/\(ASCE\)EM.1943-7889.0000564](https://doi.org/10.1061/(ASCE)EM.1943-7889.0000564).
- 1344 [42]Iourtchenko D, Mo E, Naess A. Reliability of strongly nonlinear single degree of
1345 freedom dynamic systems by the path integration method. *J. Appl. Mech.* (2008)
1346 75(6): 061016. <https://doi.org/10.1115/1.2967896>.
- 1347 [43]Di Matteo A, Di Paola M, Pirrotta A. Path integral solution for nonlinear systems
1348 under parametric Poissonian white noise input. *Prob. Eng. Mech.* (2016) 44:
1349 89-98.<https://doi.org/10.1016/j.pro bengmech.2015.09.020>.
- 1350 [44]Bucher C, Di Matteo A, Di Paola M, et al. First-passage problem for nonlinear
1351 systems under Lévy white noise through path integral method. *Nonlinear Dyn.*(2016)
1352 85(3): 1445-1456. <https://doi.org/10.1007/s11071-016-2770-9>.
- 1353 [45] Pirrotta A, Santoro R. Probabilistic response of nonlinear systems under
1354 combined normal and Poisson white noise via path integral method. *Prob. Eng. Mech.*
1355 (2011) 26(1): 26-32. <https://doi.org/10.1016/j.pro bengmech.2010.06.003>.
- 1356 [46] Zan W, Xu Y, Metzler R, et al. First-passage problem for stochastic differential
1357 equations with combined parametric Gaussian and Lévy white noises via path integral
1358 method. *J. Comput. Phys.* (2021) 435: 110264.
1359 <https://doi.org/10.1016/j.jcp.2021.110264>.
- 1360 [47]Yu J S, Cai G Q, Lin Y K. A new path integration procedure based on
1361 Gauss-Legendre scheme. *Int. J. Non-Linear Mech.* (1997) 32(4): 759-768.
1362 [https://doi.org/10.1016/S0020-7462\(96\)00096-0](https://doi.org/10.1016/S0020-7462(96)00096-0).
- 1363 [48]Kumar P, Narayanan S. Modified path integral solution of Fokker-Planck
1364 equation: Response and bifurcation of nonlinear systems. *J. Comput. Nonlinear*
1365 *Dynam.* (2010) 5(1): 011004. <https://doi.org/10.1115/1.4000312>.
- 1366 [49]Di Paola M, Santoro R. Path integral solution handled by fast Gauss transform.
1367 *Prob. Eng. Mech.* (2009) 24(3), 300-311.
1368 <https://doi.org/10.1016/j.pro bengmech.2008.07.008>.
- 1369 [50]Naess A, Johnsen J M. Response statistics of nonlinear, compliant offshore
1370 structures by the path integral solution method. *Probabilistic Engineering Mechanics*.
1371 (1993) 8(2): 91-106.[https://doi.org/10.1016/0266-8920\(93\)90003-E](https://doi.org/10.1016/0266-8920(93)90003-E).
- 1372 [51]Cai G Q, Lin Y K. Reliability of nonlinear structural frame under seismic
1373 excitation. *Journal of engineering mechanics.* (1998) 124(8):
1374 852-856.[https://doi.org/10.1061/\(ASCE\)0733-9399\(1998\)124:8\(852\)](https://doi.org/10.1061/(ASCE)0733-9399(1998)124:8(852)).
- 1375 [52]Luo J, Er G K, Iu V P. Accurate transient response of non-smoothly nonlinear
1376 system driven by multiplicative and additive modulated stochastic excitations. *Int. J.*
1377 *Nonlinear Mech.* (2024): 104768. <https://doi.org/10.1016/j.ijnonlinmec.2024.104768>.
- 1378 [53]Luo J, Er G K, Iu V P, et al. Transient probabilistic solution of stochastic
1379 oscillator under combined harmonic and modulated Gaussian white noise stimulations.
1380 *Nonlinear Dyn.* (2023) 111(19): 17709-17723.
1381 <https://doi.org/10.1007/s11071-023-08810-2>.
- 1382 [54]Bai G P, Er G K, Iu V P. Probabilistic analysis of nonlinear oscillators under
1383 correlated multi-power velocity multiplicative excitation and additive excitation.
1384 *Nonlinear Dyn.* (2024): 1-18. <https://doi.org/10.1007/s11071-024-09823-1>.
- 1385 [55]Bai G P, Ren Z X, Er G K, et al. Investigation on optimization-oriented EPC
1386 method in analyzing the non-linear oscillations under multiple excitations. *Int. J.*
1387 *Nonlinear Mech.* (2024): 104771. <https://doi.org/10.1016/j.ijnonlinmec.2024.104771>.
- 1388 [56]Guo S S, Meng F F, Shi Q. The generalized EPC method for the non-stationary
1389 probabilistic response of nonlinear dynamical system. *Probab. Eng. Mech.* (2023) 72:
1390 103420. <https://doi.org/10.1016/j.pro bengmech.2023.103420>.
- 1391 [57] Alevras P, Yurchenko D. GPU computing for accelerating the numerical Path
1392 Integration approach. *Comput. Struct.* (2016) 171: 46-53.
1393 <https://doi.org/10.1016/j.compstruc.2016.05.002>.

-
- 1394 [58] Peng J, Wang L, Wang B, et al. A path integration algorithm for stochastic
1395 dynamical systems with multiple non-smooth events. *Mech. Syst. Signal Process.*
1396 (2023) 185: 109764. <https://doi.org/10.1016/j.ymssp.2022.109764>.
- 1397 [59] Di Matteo A. Path Integral approach via Laplace's method of integration for
1398 nonstationary response of nonlinear systems. *Meccanica.* (2019) 54(9): 1351-1363.
1399 <https://doi.org/10.1007/s11012-019-00991-8>.
- 1400 [60] Di Matteo A. Response of nonlinear oscillators with fractional derivative
1401 elements under evolutionary stochastic excitations: A Path Integral approach based on
1402 Laplace's method of integration. *Prob. Eng. Mec.* (2023) 71: 103402.
1403 <https://doi.org/10.1016/j.pro bengmech.2022.103402>.
- 1404 [61] Di Matteo A, Pirrotta A. Efficient path integral approach via analytical
1405 asymptotic expansion for nonlinear systems under Gaussian white noise. *Nonlinear*
1406 *Dyn.* (2024): 1-24. <https://doi.org/10.1007/s11071-024-09822-2>.
- 1407 [62] Xu Y, Zhang H, Li Y, et al. Solving Fokker-Planck equation using deep learning.
1408 *Chaos* (2020) 30(1). <https://doi.org/10.1063/1.5132840>.
- 1409 [63] Zhang H, Xu Y, Liu Q, et al. Deep learning framework for solving Fokker-Planck
1410 equations with low-rank separation representation. *Eng. Appl. Artif. Intell.* (2023) 121:
1411 106036. <https://doi.org/10.1016/j.engappai.2023.106036>.
- 1412 [64] Wang X, Xing S, Jiang J, et al. Separable Gaussian neural networks for
1413 high-dimensional nonlinear stochastic systems. *Probab. Eng. Mech.* (2024) 76:
1414 103594. <https://doi.org/10.1016/j.pro bengmech.2024.103594>.
- 1415 [65] Chen J B, Li J. The extreme value distribution and dynamic reliability analysis of
1416 nonlinear structures with uncertain parameters. *Struct. Saf.* (2007) 29(2): 77-93.
1417 <https://doi.org/10.1016/j.strusafe.2006.02.002>.
- 1418 [66] Der Kiureghian A. The geometry of random vibrations and solutions by FORM
1419 and SORM. *Probab. Eng. Mech.* (2000) 15(1): 81-90.
1420 [https://doi.org/10.1016/S0266-8920\(99\)00011-9](https://doi.org/10.1016/S0266-8920(99)00011-9).
- 1421 [67] Bucher C. An importance sampling technique for randomly excited systems
1422 discretized by finite elements. *Proc. Int. Conf. Adv. Struct. Dyn., Hong Kong* (2000).
- 1423 [68] Qiao H W, Lyu Z Z. Dynamic reliability analysis of stochastic structures under
1424 non-stationary random excitation (in Chinese). *Chin. J. Solid Mech.* (2008) 01: 36-40.
1425 [10.19636/j.cnki.cjasm42-1250/o3.2008.01.006](https://doi.org/10.19636/j.cnki.cjasm42-1250/o3.2008.01.006).
- 1426 [69] Zhao Z, Zhao Y G, Li P P. Efficient approach for dynamic reliability analysis
1427 based on uniform design method and Box-Cox transformation. *Mech. Syst. Signal*
1428 *Process.* (2022) 172: 108967. <https://doi.org/10.1016/j.ymssp.2022.108967>.
- 1429 [70] Low Y M. A new distribution for fitting four moments and its applications to
1430 reliability analysis. *Struct. Saf.* (2013) 42: 12-25.
1431 <https://doi.org/10.1016/j.strusafe.2013.01.007>.
- 1432 [71] Zhou T, Peng Y. Adaptive Bayesian quadrature based statistical moments
1433 estimation for structural reliability analysis. *Reliab. Eng. Syst. Saf.* (2020) 198:
1434 106902. <https://doi.org/10.1016/j.ress.2020.106902>.
- 1435 [72] Zhao Y G, Zhang L W, Lu Z H, et al. First passage probability assessment of
1436 stationary non-Gaussian process using the third-order polynomial transformation. *Adv.*
1437 *Struct. Eng.* (2019) 22(1): 187-201. <https://doi.org/10.1177/1369433218782873>.
- 1438 [73] Li G, He W, Zeng Y. An improved maximum entropy method via fractional
1439 moments with Laplace transform for reliability analysis. *Struct. Multidiscip. Optim.*
1440 (2019) 59: 1301-1320. <https://doi.org/10.1007/s00158-018-2129-6>.
- 1441 [74] Xu J, Dang C. A novel fractional moments-based maximum entropy method for
1442 high-dimensional reliability analysis. *Appl. Math. Modell.* (2019) 75: 749-768.
1443 <https://doi.org/10.1016/j.apm.2019.06.037>.
- 1444 [75] Alibrandi U, Mosalam K M. Kernel density maximum entropy method with
1445 generalized moments for evaluating probability distributions, including tails, from a
1446 small sample of data. *Int. J. Numer. Methods Eng.* (2018) 113(13): 1904-1928.
1447 <https://doi.org/10.1002/nme.5725>.
- 1448 [76] Dang C, Xu J. A mixture distribution with fractional moments for efficient
1449 seismic reliability analysis of nonlinear structures. *Eng. Struct.* (2020) 208: 109912.
1450 <https://doi.org/10.1016/j.engstruct.2019.109912>.

-
- 1451 [77] Zhang L W, Lu Z H, Zhao Y G. Dynamic reliability assessment of nonlinear
1452 structures using extreme value distribution based on L-moments. *Mech. Syst. Signal*
1453 *Process.* (2021) 59: 107832. <https://doi.org/10.1016/j.ymsp.2021.107832>.
- 1454 [78] Chen Z Q, Zheng S X, Ding Z H, et al. Seismic reliability evaluation of bridges
1455 under spatially varying ground motions using a four-parameter distribution. *Eng.*
1456 *Struct.* (2021) 247: 113157. <https://doi.org/10.1016/j.engstruct.2021.113157>.
- 1457 [79] Naess A, Gaidai O. Estimation of extreme values from sampled time series.
1458 *Struct. Saf.* (2009) 31(4): 325-334. <https://doi.org/10.1016/j.strusafe.2008.06.021>.
- 1459 [80] Grigoriu M, Samorodnitsky G. Reliability of dynamic systems in random
1460 environment by extreme value theory. *Probab. Eng. Mech.* (2014) 38: 54-69.
1461 <https://doi.org/10.1016/j.pro bengmech.2014.08.005>.
- 1462 [81] Koo H, Der Kiureghian A, Fujimura K. Design-point excitation for non-linear
1463 random vibrations. *Probab. Eng. Mech.* (2005) 20(2): 136-147.
1464 <https://doi.org/10.1016/j.pro bengmech.2005.04.001>.
- 1465 [82] Breitung K. The return of the design points. *Reliab. Eng. Syst. Saf.* (2024) 247:
1466 110103. <https://doi.org/10.1016/j.res.2024.110103>.
- 1467 [83] Alibrandi U, Der Kiureghian A. A gradient-free method for determining the
1468 design point in nonlinear stochastic dynamic analysis. *Probab. Eng. Mech.* (2012) 28:
1469 2-10. <https://doi.org/10.1016/j.pro bengmech.2011.08.018>.
- 1470 [84] Fujimura K, Der Kiureghian A. Tail-equivalent linearization method for
1471 nonlinear random vibration. *Probab. Eng. Mech.* (2007) 22(1): 63-76.
1472 <https://doi.org/10.1016/j.pro bengmech.2006.08.001>.
- 1473 [85] Broccardo M, Alibrandi U, Wang Z, et al. The tail equivalent linearization
1474 method for nonlinear stochastic processes, genesis and developments. *Risk Reliab.*
1475 *Anal. Theory Appl.* (2017): 109-142. https://doi.org/10.1007/978-3-319-52425-2_6.
- 1476 [86] Garrè L, Der Kiureghian A. Tail-equivalent linearization method in frequency
1477 domain and application to Mar. *Struct. Mar. Struct.* (2010) 23(3): 322-338.
1478 <https://doi.org/10.1016/j.marstruc.2010.07.006>.
- 1479 [87] Broccardo M, Der Kiureghian A. Multicomponent nonlinear stochastic dynamic
1480 analysis by tail-equivalent linearization. *J. Eng. Mech.* (2016) 142(3): 04015100.
1481 [https://doi.org/10.1061/\(ASCE\)EM.1943-7889.0001026](https://doi.org/10.1061/(ASCE)EM.1943-7889.0001026).
- 1482 [88] Wang Z, Der Kiureghian A. Tail-equivalent linearization of inelastic
1483 multisupport structures subjected to spatially varying stochastic ground motion. *J. Eng.*
1484 *Mech.* (2016) 142(8): 04016053.
1485 [https://doi.org/10.1061/\(ASCE\)EM.1943-7889.000110](https://doi.org/10.1061/(ASCE)EM.1943-7889.000110).
- 1486 [89] Raoufi R, Ghafory-Ashtiany M. Nonlinear random vibration using updated tail
1487 equivalent linearization method. *Int. J. Adv. Struct. Eng.* (2014) 6: 1-12.
1488 <https://doi.org/10.1007/s40091-014-0045-6>.
- 1489 [90] Raoufi R, Ghafory-Ashtiany M. Random vibration of nonlinear structures with
1490 stiffness and strength deterioration by modified tail equivalent linearization method.
1491 *Earthq. Eng. Eng. Vib.* (2019) 18: 597-610.
1492 <https://doi.org/10.1007/s11803-019-0524-7>.
- 1493 [91] Chen D, Dang C, Valdebenito M A, et al. First-passage probability estimation of
1494 high-dimensional nonlinear stochastic dynamic systems by a fractional
1495 moments-based mixture distribution approach. *Mech. Syst. Signal Process.* (2023)
1496 185: 109775. <https://doi.org/10.1016/j.ymsp.2022.109775>.
- 1497 [92] Broccardo M, Der Kiureghian A. Nonlinear stochastic dynamic analysis by
1498 evolutionary tail-equivalent linearization method. *Struct. Saf.* (2021) 90: 102044.
1499 <https://doi.org/10.1016/j.strusafe.2020.102044>.
- 1500 [93] Chen J B, Lyu M Z. A new approach for time-variant probability density
1501 function of the maximal value of stochastic dynamical systems. *J. Comput. Phys.*
1502 (2020) 415: 109525. <https://doi.org/10.1016/j.jcp.2020.109525>.
- 1503 [94] Lyu M Z, Chen J B, Pirrotta A. A novel method based on augmented Markov
1504 vector process for the time-variant extreme value distribution of stochastic dynamical
1505 systems enforced by Poisson white noise. *Commun. Nonlinear Sci. Numer. Simul.*
1506 (2020) 80: 104974. <https://doi.org/10.1016/j.cnsns.2019.104974>.

-
- 1507 [95] Weng Y Y, Zhang X Y, Lu Z H, et al. A conditional extreme value distribution
1508 method for dynamic reliability analysis of stochastic structures. *Struct. Saf.* (2024)
1509 106: 102398. <https://doi.org/10.1016/j.strusafe.2023.102398>.
- 1510 [96] Dang C, Wei P, Beer M. An approach to evaluation of EVD and small failure
1511 probabilities of uncertain nonlinear structures under stochastic seismic excitations.
1512 *Mech. Syst. Signal Process.* (2021) 152: 107468.
1513 <https://doi.org/10.1016/j.ymssp.2020.107468>.
- 1514 [97] Chen J B, Li J. Dynamic response and reliability analysis of nonlinear stochastic
1515 structures. *Probab. Eng. Mech.* (2005) 20(1): 33-44.
1516 <https://doi.org/10.1016/j.probengmech.2004.05.006>.
- 1517 [98] Li J, Chen J B, Fan W L. The equivalent extreme-value event and evaluation of
1518 the structural system reliability. *Struct. Saf.* (2007) 29(2): 112-131.
1519 <https://doi.org/10.1016/j.strusafe.2006.03.002>.
- 1520 [99] Li J, Chen J B. The principle of preservation of probability and the generalized
1521 density evolution equation. *Struct. Saf.* (2008) 30(1): 65-77.
1522 <https://doi.org/10.1016/j.strusafe.2006.08.001>.
- 1523 [100] Li J, Chen J B. Mapping dimensionality reduction methods for the analysis
1524 of density evolution in random structural response (in Chinese). *Chin. J. Theor. Appl.*
1525 *Mech.* (2005) 04: 460-466. <https://doi.org/10.3321/j.issn:0459-1879.2005.04.011>.
- 1526 [101] Chen J B, Li J. The Strategy of selecting points via sphere of contact in
1527 probability density evolution method for response analysis of stochastic structures (in
1528 Chinese). *J. Vibr. Eng.* (2006) 01: 1-8.
1529 <https://doi.org/10.16385/j.cnki.issn.1004-4523.2006.01.001>.
- 1530 [102] Chen J B, Li J. The strategy of selecting points via number theoretical in
1531 probability density evolution method for response analysis of stochastic structures (in
1532 Chinese). *Chin. J. Theor. Appl. Mech.* (2006) 38(1): 134-140.
- 1533 [103] Chen J B, Zhang S H. Improving point selection in cubature by a new
1534 discrepancy. *SIAM J. Sci. Comput.* (2013) 35(5): A2121-A2149.
1535 <https://doi.org/10.1137/12089377X>.
- 1536 [104] Chen J B, Ghanem R, Li J. Partition of the probability-assigned space in
1537 probability density evolution analysis of nonlinear stochastic structures. *Probab. Eng.*
1538 *Mech.* (2009) 24(1): 27-42. <https://doi.org/10.1016/j.probengmech.2007.12.017>.
- 1539 [105] Chen J B, Yang J Y, Li J. A GF-discrepancy for point selection in stochastic
1540 seismic response analysis of structures with uncertain parameters. *Struct. Saf.* (2016)
1541 59: 20-31. <https://doi.org/10.1016/j.strusafe.2015.11.001>.
- 1542 [106] Shi S, Du D S, Wang S G, et al. Non-uniform time step TVD scheme for
1543 probability density evolution function with improvement of initial condition. *Chin. J.*
1544 *Theor. Appl. Mech.* (2019) 51(4): 1223-1234.
1545 <https://doi.org/10.6052/0459-1879-18-446>.
- 1546 [107] Papadopoulos V, Kalogeris I. A Galerkin-based formulation of the
1547 probability density evolution method for general stochastic finite element systems.
1548 *Comput. Mech.* (2016) 57(5): 701-716. <https://doi.org/10.1007/s00466-015-1256-9>.
- 1549 [108] Fan W L, Li J. The δ -function sequence solution for the generalized density
1550 evolution equation (in Chinese). *Chin. J. Theor. Appl. Mech.* (2009) 41(03): 398-409.
1551 <https://doi.org/10.1007/s00466-008-0345-4>.
- 1552 [109] Fan W L, Zhang C T, Li Z L. Error analysis of solution for generalized
1553 density evolution equation via a family of δ sequences (in Chinese). *Eng. Mech.*
1554 (2013) 30(06): 1-5.
- 1555 [110] Tao W F, Li J. A Difference-Wavelet method for solving generalized
1556 density evolution equation in stochastic structural analysis. *Int. J. Struct. Stab. Dyn.*
1557 (2017) 1750055. <https://doi.org/10.1142/S0219455417500559>.
- 1558 [111] Tao W F, Li J. An ensemble evolution numerical method for solving
1559 generalized density evolution equation. *Probab. Eng. Mech.* (2017) 48: 1-11.
1560 <https://doi.org/10.1016/j.probengmech.2017.03.001>.
- 1561 [112] Li J, Chen J B, Sun W L, et al. Advances of the probability density
1562 evolution method for nonlinear stochastic systems. *Probab. Eng. Mech.* (2012) 28(4):
1563 132-142. <https://doi.org/10.1016/j.probengmech.2011.08.019>.

-
- 1564 [113] Li J, Sun W L. The refined algorithm of generalized density evolution
1565 equation based on reproducing kernel particle method (in Chinese). *Chin. J. Comput.*
1566 *Mech.* (2016) 33(04): 543-548+587. <https://doi.org/10.7511/jslx201604019>.
- 1567 [114] Chen G H, Yang D X. Direct probability integral method for stochastic
1568 response analysis of static and dynamic structural systems. *Comput. Methods Appl.*
1569 *Mech. Eng.* (2019) 357: 112612. <https://doi.org/10.1016/j.cma.2019.112612>.
- 1570 [115] Chen G H, Yang D X, Liu Y H, et al. System reliability analyses of static
1571 and dynamic structures via direct probability integral method. *Comput. Methods Appl.*
1572 *Mech. Eng.* (2022) 388: 114262. <https://doi.org/10.1016/j.cma.2021.114262>.
- 1573 [116] Chen G H, Yang D X. A unified analysis framework of static and dynamic
1574 structural reliabilities based on direct probability integral method. *Mech. Syst. Signal*
1575 *Process.* (2021) 158: 107783. <https://doi.org/10.1016/j.ymsp.2021.107783>.
- 1576 [117] Li X L, Chen G H, Cui H C, et al. Direct probability integral method for
1577 static and dynamic reliability analysis of structures with complicated performance
1578 functions. *Comput. Methods Appl. Mech. Eng.* (2021) 374(1): 113583.
1579 <https://doi.org/10.1016/j.cma.2020.113583>.
- 1580 [118] Meng Z, Zhao J, Chen G, et al. Hybrid uncertainty propagation and
1581 reliability analysis using direct probability integral method and exponential convex
1582 model. *Reliab. Eng. Syst. Saf.* (2022) 228: 108803.
1583 <https://doi.org/10.1016/j.ress.2022.108803>.
- 1584 [119] Zhou Y, Jing M Y, Pang R, et al. A novel method for the dynamic
1585 reliability analysis of slopes considering dependent random parameters via the direct
1586 probability integral method. *Struct.* (2022) 43: 1732-1749.
1587 <https://doi.org/10.1016/j.istruc.2022.07.074>.
- 1588 [120] Wang D, Li J. A reproducing kernel particle method for solving generalized
1589 probability density evolution equation in stochastic dynamic analysis. *Comput. Mech.*
1590 (2020) 65: 597-607. <https://doi.org/10.1007/s00466-019-01785-1>.
- 1591 [121] Wang D, Sun W, Li J. An RKPM-based formulation of the generalized
1592 probability density evolution equation for stochastic dynamic systems. *Probab. Eng.*
1593 *Mech.* (2021) 66: 103152. <https://doi.org/10.1016/j.probengmech.2021.103152>.
- 1594 [122] Das S, Tesfamariam S. Reliability analysis of structures using probability
1595 density evolution method and stochastic spectral embedding surrogate model. *Earthq.*
1596 *Eng. Struct. Dyn.* (2023) 52(8): 2480-2497. <https://doi.org/10.1002/eqe.3835>.
- 1597 [123] Xu Y Z. Probability density evolution filter. *Probab. Eng. Mech.* (2022) 69:
1598 103325. <https://doi.org/10.1016/j.probengmech.2022.103325>.
- 1599 [124] Zhang H, Xu Y Z. Chebyshev collocation based sequential matrix
1600 exponential method for the generalized density evolution equation. *Probab. Eng.*
1601 *Mech.* (2021) 63: 103118. <https://doi.org/10.1016/j.probengmech.2021.103118>.
- 1602 [125] Feng D C, Cao X Y, Beer M. An enhanced PDEM-based framework for
1603 reliability analysis of structures considering multiple failure modes and limit states.
1604 *Probab. Eng. Mech.* (2022) 70: 103367.
1605 <https://doi.org/10.1016/j.probengmech.2022.103367>.
- 1606 [126] Lyu M Z, Chen J B. First-passage reliability of high-dimensional nonlinear
1607 systems under additive excitation by the ensemble-evolving-based generalized density
1608 evolution equation. *Probab. Eng. Mech.* (2021) 63: 103119.
1609 <https://doi.org/10.1016/j.probengmech.2021.103119>.
- 1610 [127] Sun T T, Lyu M Z, Chen J B. Property of intrinsic drift coefficients in
1611 globally-evolving-based generalized density evolution equation for the first-passage
1612 reliability assessment. *Acta Mech. Sin.* (2023) 39: 722471.
1613 <https://doi.org/10.1007/s10409-023-22471-x>.
- 1614 [128] Lyu M Z, Chen J B. A unified formalism of the GE-GDEE for generic
1615 continuous responses and first-passage reliability analysis of multi-dimensional
1616 nonlinear systems subjected to non-white-noise excitations. *Struct. Saf.* (2022) 98:
1617 102233. <https://doi.org/10.1016/j.strusafe.2022.102233>.
- 1618 [129] Lyu M Z, Chen J B. Probability density global evolution method for
1619 reliability analysis of high-dimensional nonlinear systems under non-stationary
1620 random excitations (in Chinese). *J. Vibr. Eng.* (2022) 1: 1-13.
1621 [10.16385/j.cnki.issn.1004-4523.2024.06.001](https://doi.org/10.16385/j.cnki.issn.1004-4523.2024.06.001).

-
- 1622 [130] Sun T T, Chen J B. Physically driven exact dimension reduction of a class
1623 of nonlinear multidimensional systems subjected to additive white noise.
1624 ASCE-ASME J. Risk Uncert. Eng. Syst. (2022) 8(2): 04022012.
1625 <https://doi.org/10.1061/AJRUA6.0001229>.
- 1626 [131] Luo Y, Chen J, Spanos P D. Determination of monopile offshore structure
1627 response to stochastic wave loads via analog filter approximation and GV-GDEE
1628 procedure. Probab. Eng. Mech. (2022) 67: 103197.
1629 <https://doi.org/10.1016/j.probengmech.2022.103197>.
- 1630 [132] Chen J B, Lyu M Z. Globally-evolving-based generalized density evolution
1631 equation for nonlinear systems involving randomness from both system parameters
1632 and excitations. Proc. R. Soc. A (2022) 478(2264): 20220356.
1633 <https://doi.org/10.1098/rspa.2022.0356>.
- 1634 [133] Luo Y, Spanos P D, Chen J. Stochastic response determination of
1635 multi-dimensional nonlinear systems endowed with fractional derivative elements by
1636 the GE-GDEE. Int. J. Nonlinear Mech. (2022) 147: 104247.
1637 <https://doi.org/10.1016/j.ijnonlinmec.2022.104247>.
- 1638 [134] Xu J, Zhang W, Sun R. Efficient reliability assessment of structural
1639 dynamic systems with unequal weighted quasi-Monte Carlo simulation. Comput.
1640 Struct. (2016) 175: 37-51. <https://doi.org/10.1016/j.compstruc.2016.06.005>.
- 1641 [135] Sichani M T, Nielsen S R K, Bucher C. Applications of asymptotic
1642 sampling on high dimensional structural dynamic problems. Struct. Saf. (2011)
1643 33(4-5): 305-316. <https://doi.org/10.1016/j.strusafe.2011.05.002>.
- 1644 [136] Koo H, Der Kiureghian A. Importance sampling of first-excursion
1645 probability for nonlinear systems. Int. Conf. Appl. Stat. Prob. Civ. Eng. (2003).
- 1646 [137] Olsen A I, Naess A. An importance sampling procedure for estimating
1647 failure probabilities of non-linear dynamic systems subjected to random noise. Int. J.
1648 Nonlinear Mech. (2007) 42(6): 848-863.
1649 <https://doi.org/10.1016/j.ijnonlinmec.2007.03.014>.
- 1650 [138] Bayer V, Bucher C. Importance sampling for first passage problems of
1651 nonlinear structures. Probab. Eng. Mech. (1999) 14(1-2): 27-32.
1652 [https://doi.org/10.1016/S0266-8920\(98\)00014-9](https://doi.org/10.1016/S0266-8920(98)00014-9).
- 1653 [139] Valdebenito M A, Jensen H A, Labarca A A. Estimation of first excursion
1654 probabilities for uncertain stochastic linear systems subject to Gaussian load. Comput.
1655 Struct. (2014) 138: 36-48. <https://doi.org/10.1016/j.compstruc.2014.02.010>.
- 1656 [140] Au S K, Beck J L. Importance sampling in high dimensions. Struct. Saf.
1657 (2003) 25(2): 139-163. [https://doi.org/10.1016/S0167-4730\(02\)00047-4](https://doi.org/10.1016/S0167-4730(02)00047-4).
- 1658 [141] Papaioannou I, Papadimitriou C, Straub D. Sequential importance sampling
1659 for structural reliability analysis. Struct. Saf. (2016) 62: 66-75.
1660 <https://doi.org/10.1016/j.strusafe.2016.06.002>.
- 1661 [142] Au S K, Beck J L. Subset Simulation and its Application to seismic risk
1662 based on dynamic analysis. J. Eng. Mech. (2003) 129(8): 901-917.
1663 [https://doi.org/10.1061/\(ASCE\)0733-9399\(2003\)129:8\(901\)](https://doi.org/10.1061/(ASCE)0733-9399(2003)129:8(901)).
- 1664 [143] Au S K, Beck J L. Estimation of small failure probabilities in high
1665 dimensions by subset simulation. Probab. Eng. Mech. (2001) 16(4): 263-277.
1666 [https://doi.org/10.1016/S0266-8920\(01\)00019-4](https://doi.org/10.1016/S0266-8920(01)00019-4).
- 1667 [144] Zuev K M, Beck J L, Au S K, et al. Bayesian post-processor and other
1668 enhancements of Subset Simulation for estimating failure probabilities in high
1669 dimensions. Comput. Struct. (2012) 92: 283-296.
1670 <https://doi.org/10.1016/j.compstruc.2011.10.017>.
- 1671 [145] Liu P, Yao Q F. Dynamic reliability computation for non-linear structures
1672 based on subset simulation method (in Chinese). Eng. Mech. (2010) 12: 1-5.
1673 CNKI:SUN.0.2010-12-014.
- 1674 [146] Katafygiotis L S, Cheung S H. Application of spherical subset simulation
1675 method and auxiliary domain method on a benchmark reliability study. Struct. Saf.
1676 (2007) 29(3): 194-207. <https://doi.org/10.1016/j.strusafe.2006.07.003>.
- 1677 [147] Song S F, Lyu Z Z. A high-precision and effective sequential Importance
1678 Sampling method for reliability analysis of small failure probability in high

1679 dimensions (in Chinese). *J. Northw. Polytech. Univ.* (2006) (06): 782-786.
1680 10.3969/j.issn.1000-2758.2006.06.024.

1681 [148] Katafygiotis L S, Cheung S H. Domain decomposition method for
1682 calculating the failure probability of linear dynamic systems subjected to Gaussian
1683 stochastic loads. *J. Eng. Mech.* (2006) 132(5): 475-486.
1684 [https://doi.org/10.1061/\(ASCE\)0733-9399\(2006\)132:5\(475\)](https://doi.org/10.1061/(ASCE)0733-9399(2006)132:5(475)).

1685 [149] Su C, Xu R. Time-domain method for dynamic reliability of structural
1686 systems subject to non-stationary random excitations (in Chinese). *Chin. J. Theor.*
1687 *Appl. Mech.* (2010) 42(03): 512-520. 10.6052/0459-1879-2010-3-2009-042.

1688 [150] Xu R, Zhang J X, Su C. Time-domain explicit formulation Subset
1689 Simulation method for dynamic reliability subject to non-stationary random
1690 excitations (in Chinese). *Eng. Mech.* (2013) 30(07): 28-33+39.
1691 10.6052/j.issn.1000-4750.2012.03.0210.

1692 [151] Pradlwarter H J, Schueller G I, Koutsourelakis P S, et al. Application of line
1693 sampling simulation method to reliability benchmark problems. *Struct. Saf.* (2007)
1694 29(3): 208-221. <https://doi.org/10.1016/j.strusafe.2006.07.009>.

1695 [152] Angelis M D, Patelli E, Beer M. Advanced line sampling for efficient
1696 robust reliability analysis. *Struct. Saf.* (2015) 52: 170-182.
1697 <https://doi.org/10.1016/j.strusafe.2014.10.002>.

1698 [153] Papaioannou I, Straub D. Combination line sampling for structural
1699 reliability analysis. *Struct. Saf.* (2021) 88: 102025.
1700 <https://doi.org/10.1016/j.strusafe.2020.102025>.

1701 [154] Nie J, Ellingwood B R. Directional methods for structural reliability
1702 analysis. *Struct. Saf.* (2000) 22(3): 233-249.
1703 [https://doi.org/10.1016/S0167-4730\(00\)00014-X](https://doi.org/10.1016/S0167-4730(00)00014-X).

1704 [155] Ditlevsen O, Melchers RE, Gluwer H. General multi dimensional probability
1705 integration by directional simulation. *Comput. Struct.* (1990) 36(2): 355-368.
1706 [https://doi.org/10.1016/0045-7949\(90\)90134-N](https://doi.org/10.1016/0045-7949(90)90134-N).

1707 [156] Bjerager P. Probability integration by directional simulation. *J. Eng. Mech.*
1708 (1988) 114(8): 1285-1302.
1709 [https://doi.org/10.1061/\(ASCE\)0733-9399\(1988\)114:8\(1285\)](https://doi.org/10.1061/(ASCE)0733-9399(1988)114:8(1285)).

1710 [157] Grooteman F. An adaptive directional importance sampling method for
1711 structural reliability. *Probab. Eng. Mech.* (2011) 26(2): 134-141.
1712 <https://doi.org/10.1016/j.probengmech.2010.11.002>.

1713 [158] Zhang X, Lu Z, and Cheng K. Cross-entropy-based directional importance
1714 sampling with von mises-fisher mixture model for reliability analysis. *Reliab. Eng.*
1715 *Syst. Saf.* (2022) 220: 108306. <https://doi.org/10.1016/j.ress.2021.108306>.

1716 [159] Cheng K, Papaioannou I, Lu Z, Zhang X, Wang YP. Rare event estimation
1717 with sequential directional importance sampling. *Struct. Saf.* (2023) 100: 102291.
1718 <https://doi.org/10.1016/j.strusafe.2022.102291>.

1719 [160] Kurtz N, Song J. Cross-entropy-based adaptive importance sampling using
1720 gaussian mixture. *Struct. Saf.* (2013) 42: 35-44.
1721 <https://doi.org/10.1016/j.strusafe.2013.01.006>.

1722 [161] Wang Z, Song J. Cross-entropy-based adaptive importance sampling using
1723 von mises-fisher mixture for high dimensional reliability analysis. *Struct. Saf.* (2016)
1724 59: 42-52. <https://doi.org/10.1016/j.strusafe.2015.11.002>.

1725 [162] Papaioannou I, Geyer S, Straub D. Improved cross entropy-based importance
1726 sampling with a flexible mixture model. *Reliab. Eng. Syst. Saf.* (2019) 191: 106564.
1727 <https://doi.org/10.1016/j.ress.2019.106564>.

1728 [163] Gasser C, Bucher C. An optimized strategy for using asymptotic sampling
1729 for reliability analysis. *Struct. Saf.* (2018) 71: 33-40.
1730 <https://doi.org/10.1016/j.strusafe.2017.11.002>.

1731 [164] Xu J, Kong F. A new unequal-weighted sampling method for efficient
1732 reliability analysis. *Reliab. Eng. Syst. Saf.* (2018) 172: 94-102.
1733 <https://doi.org/10.1016/j.ress.2017.12.007>.

1734 [165] Kanjilal O, Papaioannou I, Straub D. Cross entropy-based importance
1735 sampling for first-passage probability estimation of randomly excited linear structures

1736 with parameter uncertainty. *Struct. Saf.* (2021) 91: 102090.
1737 <https://doi.org/10.1016/j.strusafe.2021.102090>.

1738 [166] Kanjilal O, Papaioannou I, Straub D. Series system reliability of uncertain
1739 linear structures under Gaussian excitation by cross entropy-based importance
1740 sampling. *J. Eng. Mech.* (2022) 148(1): 04021136.
1741 [https://doi.org/10.1061/\(ASCE\)EM.1943-7889.0002015](https://doi.org/10.1061/(ASCE)EM.1943-7889.0002015).

1742 [167] Misraji M A, Valdebenito M A, Jensen H A, et al. Application of directional
1743 importance sampling for estimation of first excursion probabilities of linear structural
1744 systems subject to stochastic Gaussian loading. *Mech. Syst. Signal Process.* (2020)
1745 139: 106621. <https://doi.org/10.1016/j.ymsp.2020.106621>.

1746 [168] Behrendt M, Lyu M Z, Luo Y, et al. Failure probability estimation of
1747 dynamic systems employing relaxed power spectral density functions with dependent
1748 frequency modeling and sampling. *Probab. Eng. Mech.* (2024): 103592.
1749 <https://doi.org/10.1016/j.probengmech.2024.103592>.

1750 [169] Yuan X, Liu S, Faes M G R, et al. An efficient importance sampling
1751 approach for reliability analysis of time-variant structures subject to time-dependent
1752 stochastic load. *Mech. Syst. Signal Process.* (2021) 159: 107699.
1753 <https://doi.org/10.1016/j.ymsp.2021.107699>.

1754 [170] Yuan X, Zheng W, Zhao C, et al. Line sampling for time-variant failure
1755 probability estimation using an adaptive combination approach. *Reliab. Eng. Syst. Saf.*
1756 (2024) 243: 109885. <https://doi.org/10.1016/j.ress.2023.109885>.

1757 [171] Buratti N, Ferracuti B, Savoia M. Response surface with random factors for
1758 seismic fragility of reinforced concrete frames. *Struct. Saf.* (2010) 32(1): 42-51.
1759 <https://doi.org/10.1016/j.strusafe.2009.06.003>.

1760 [172] Gavin H P, Yau S C. High-order limit state functions in the response surface
1761 method for structural reliability analysis. *Struct. Saf.* (2008) 30(2): 162-179.
1762 <https://doi.org/10.1016/j.strusafe.2006.10.003>.

1763 [173] Qiao H W, Lyu Z Z, Guan A R, et al. Dynamic reliability analysis of
1764 stochastic structures under stationary random excitation using the Hermite
1765 polynomials approximation (in Chinese). *Eng. Mech.* (2009) 26(02): 60-64.
1766 CNKI:SUN.0.2009-02-014.

1767 [174] Yang J, Ma M P, Wang X. Conditional probability method for dynamic
1768 reliability calculation of random structures (in Chinese). *Eng. Mech.* (2018) 35(01): 5.
1769 [10.6052/j.issn.1000-4750.2017.05.S030](https://doi.org/10.6052/j.issn.1000-4750.2017.05.S030).

1770 [175] Wang X, Jiang J, Hong L, et al. First-passage problem in random vibrations
1771 with radial basis function neural networks. *J. Vibr. Acoust.* (2022) 144(5): 051014.
1772 <https://doi.org/10.1115/1.4054437>.

1773 [176] Lu C, Feng Y W, Liem R P, et al. Improved Kriging with extremum response
1774 surface method for structural dynamic reliability and sensitivity analyses. *Aerosp. Sci.*
1775 *Technol.* (2018) 76: 164-175. <https://doi.org/10.1016/j.ast.2018.02.012>.

1776 [177] Jia B Y, Yu X L, Yan Q H, et al. Seismic dynamic reliability analysis of
1777 bridges based on ISC-Kriging Response Surface method (in Chinese). *J. South China*
1778 *Univ. Technol.* (2015) (10): 1-10. [10.3969/j.issn.1000-565X.2015.10.009](https://doi.org/10.3969/j.issn.1000-565X.2015.10.009).

1779 [178] Papadopoulos V, Giovanis D, Lagaros N, Papadrakakis M. Accelerated
1780 subset simulation with neural networks for reliability analysis, *Comput. Methods*
1781 *Appl. Mech. Eng.* (2012) 223-224: 70-80. <https://doi.org/10.1016/j.cma.2012.02.013>.

1782 [179] Angelikopoulos P, Papadimitriou C, Koumoutsakos P. X-TMCMC:
1783 Adaptive kriging for Bayesian inverse modeling. *Comput. Methods Appl. Mech. Eng.*
1784 (2015) 289: 409-428. <https://doi.org/10.1016/j.cma.2015.01.015>.

1785 [180] Giovanis DG, Papaioannou I, Straub D, Papadopoulos V. Bayesian updating
1786 with subset simulation using artificial neural networks. *Comput. Methods Appl. Mech.*
1787 *Engrg.* (2017) 319: 124-145. <https://doi.org/10.1016/j.cma.2017.02.025>.

1788 [181] Moustapha M, Marelli S, Sudret B. Active learning for structural reliability:
1789 Survey, general framework and benchmark. *Struct. Saf.* (2022) 96: 102174.
1790 <https://doi.org/10.1016/j.strusafe.2021.102174>.

1791 [182] Jiang Y B, Luo J, Liao G, et al. An efficient method for generation of
1792 uniform support vector and its application in structural failure function fitting. *Struct.*
1793 *Saf.* (2015) 54: 1-9. <https://doi.org/10.1016/j.strusafe.2014.12.004>.

-
- 1794 [183] Jiang Y B, Zhao L J, Beer M, et al. Multiple response surfaces method with
1795 advanced classification of samples for structural failure function fitting. *Struct. Saf.*
1796 (2017) 64: 87-97. <https://doi.org/10.1016/j.strusafe.2016.10.002>.
- 1797 [184] Jiang Y B, Zhao L J, Beer M, et al. Dominant failure mode analysis using
1798 representative samples obtained by multiple response surfaces method. *Probab. Eng.*
1799 *Mech.* (2020) 59: 103005. <https://doi.org/10.1016/j.probengmech.2019.103005>.
- 1800 [185] Kim J, Yi S, Song J. Estimation of first-passage probability under stochastic
1801 wind excitations by active-learning-based heteroscedastic Gaussian process. *Struct.*
1802 *Saf.* (2023) 100: 102268. <https://doi.org/10.1016/j.strusafe.2022.102268>.
- 1803 [186] Huang H, Wang H, Li Y, et al. Small failure probability analysis of
1804 stochastic structures based on a new hybrid approach. *Probab. Eng. Mech.* (2024) 76:
1805 103611. <https://doi.org/10.1016/j.probengmech.2024.103611>.
- 1806 [187] Kougioumtzoglou I A, Spanos P D. An analytical Wiener path integral
1807 technique for non-stationary response determination of nonlinear oscillators. *Probab.*
1808 *Eng. Mech.* (2012) 28: 125-131. <https://doi.org/10.1016/j.probengmech.2011.08.022>.
- 1809 [188] Psaros A F, Zhao Y, Kougioumtzoglou I A. An exact closed-form solution
1810 for linear multi-degree-of-freedom systems under Gaussian white noise via the
1811 Wiener path integral technique. *Probab. Eng. Mech.* (2020) 60: 103040.
1812 <https://doi.org/10.1016/j.probengmech.2020.103040>.
- 1813 [189] Kougioumtzoglou I A, Spanos P D. Nonstationary stochastic response
1814 determination of nonlinear systems: A Wiener path integral formalism. *J. Eng. Mech.*
1815 (2014) 140(9): 76-82. [https://doi.org/10.1061/\(ASCE\)EM.1943-7889.0000780](https://doi.org/10.1061/(ASCE)EM.1943-7889.0000780).
- 1816 [190] Psaros A F, Petromichelakis I, Kougioumtzoglou I A. Wiener path integrals
1817 and multi-dimensional global bases for non-stationary stochastic response
1818 determination of structural systems. *Mech. Syst. Signal Process.* (2019) 128: 551-571.
1819 <https://doi.org/10.1016/j.ymsp.2019.04.014>.
- 1820 [191] Petromichelakis I, Kougioumtzoglou I A. Addressing the curse of
1821 dimensionality in stochastic dynamics: A Wiener path integral variational formulation
1822 with free boundaries. *Proc. R. Soc. A* (2020) 476: 2243.
1823 <https://doi.org/10.1098/rspa.2020.0385>.
- 1824 [192] Di Matteo A, Kougioumtzoglou I A, Pirrotta A, et al. Stochastic response
1825 determination of nonlinear oscillators with fractional derivatives elements via the
1826 Wiener path integral. *Probab. Eng. Mech.* (2014) 38: 127-135.
1827 <https://doi.org/10.1016/j.probengmech.2014.07.001>.
- 1828 [193] Zhang Y, Kougioumtzoglou I A, Kong F. A Wiener path integral technique
1829 for determining the stochastic response of nonlinear oscillators with fractional
1830 derivative elements: A constrained variational formulation with free boundaries.
1831 *Probab. Eng. Mech.* (2023) 71: 103410.
1832 <https://doi.org/10.1016/j.probengmech.2022.103410>.
- 1833 [194] Psaros A F, Kougioumtzoglou I A, Petromichelakis I. Sparse
1834 representations and compressive sampling for enhancing the computational efficiency
1835 of the Wiener path integral technique. *Mech. Syst. Signal Process.* (2018) 111: 87-101.
1836 <https://doi.org/10.1016/j.ymsp.2018.03.056>.
- 1837 [195] Mavromatis I G, Kougioumtzoglou I A, Spanos P D. An extrapolation
1838 approach within the Wiener path integral technique for efficient stochastic response
1839 determination of nonlinear systems. *Int. J. Nonlinear Mech.* (2024) 160: 104646.
1840 <https://doi.org/10.1016/j.ijnonlinmec.2024.104646>.
- 1841 [196] Dang C, Valdebenito M A, Faes M G R, et al. Structural reliability analysis:
1842 A Bayesian perspective. *Struct. Saf.* (2022) 99: 102259.
1843 <https://doi.org/10.1016/j.strusafe.2022.102259>.
- 1844 [197] Dang C, Wei P, Song J, et al. Estimation of failure probability function
1845 under imprecise probabilities by active learning - augmented probabilistic integration.
1846 *ASCE-ASME J. Risk Uncert. Eng. Syst., Part A: Civ. Eng.* (2021) 7(4): 04021054.
1847 <https://doi.org/10.1061/AJRUA6.0001179>.
- 1848 [198] Dang C, Wei P, Faes M G R, et al. Parallel adaptive Bayesian quadrature
1849 for rare event estimation. *Reliab. Eng. Syst. Saf.* (2022) 225: 108621.
1850 <https://doi.org/10.1016/j.ress.2022.108621>.

-
- 1851 [199] Dang C, Matthias G. R, Valdebenito M A, et al. Partially Bayesian active
1852 learning cubature for structural reliability analysis with extremely small failure
1853 probabilities. *Comput. Methods Appl. Mech. Eng.* (2024) 422: 116828.
1854 <https://doi.org/10.1016/j.cma.2024.116828>.
- 1855 [200] Dang C, Beer M. Semi-Bayesian active learning quadrature for estimating
1856 extremely low failure probabilities. *Reliab. Eng. Syst. Saf.* (2024) 246: 110052.
1857 <https://doi.org/10.1016/j.ress.2024.110052>.
- 1858 [201] Hu Z, Dang C, Wang L, et al. Parallel Bayesian probabilistic integration for
1859 structural reliability analysis with small failure probabilities. *Struct. Saf.* (2024) 106:
1860 102409.
- 1861 [202] Dang C, Cicirello A, Valdebenito M A, et al. Structural reliability analysis
1862 with extremely small failure probabilities: A quasi-Bayesian active learning method.
1863 *Probab. Eng. Mech.* (2024) 76: 103613.
1864 <https://doi.org/10.1016/j.probengmech.2024.103613>.
- 1865 [203] Dang C, Zhou T, Valdebenito M A, et al. Yet another Bayesian active
1866 learning reliability analysis method. *Struct. Saf.* (2024) (In Press): 102539.
1867 <https://doi.org/10.1016/j.strusafe.2024.102539>.
- 1868 [204] Jiang X, Lu Z. Adaptive Kriging-based Bayesian updating of model and
1869 reliability. *Struct. Saf.* (2023) 104: 102362.
1870 <https://doi.org/10.1016/j.strusafe.2023.102362>.
- 1871 [205] Dang C, Valdebenito M A, Wei P F, et al. Bayesian active learning line
1872 sampling with log-normal process for rare-event probability estimation. *Reliab. Eng.*
1873 *Syst. Saf.* (2024) 246: 110053. <https://doi.org/10.1016/j.ress.2024.110053>.
- 1874 [206] Dang C, Valdebenito M A, Matthias G. R, et al. Structural reliability
1875 analysis by line sampling: A Bayesian active learning treatment. *Struct. Saf.* (2023)
1876 104: 102351. <https://doi.org/10.1016/j.strusafe.2023.102351>.
- 1877 [207] Faes M G R, Valdebenito M A, Yuan X, et al. Augmented reliability
1878 analysis for estimating imprecise first excursion probabilities in stochastic linear
1879 dynamics. *Adv. Eng. Softw.* (2021) 155: 102993.
1880 <https://doi.org/10.1016/j.advengsoft.2021.102993>.
- 1881 [208] Papadimitriou C, Beck JL, Katafygiotis LS. Updating robust reliability using
1882 structural test data. *Probab. Eng. Mech.*(2001) 16(2): 103-113.
1883 [https://doi.org/10.1016/S0266-8920\(00\)00012-6](https://doi.org/10.1016/S0266-8920(00)00012-6).
- 1884 [209] Ni YQ, Chen R. Strain monitoring based bridge reliability assessment using
1885 parametric Bayesian mixture model. *Eng. Struct.*(2021) 226: 111406.
1886 <https://doi.org/10.1016/j.engstruct.2020.111406>.
- 1887 [210] Beck JL, Au SK. Bayesian updating of structural models and reliability using
1888 Markov chain Monte Carlo simulation. *J. Eng. Mech.* (2002) 128(4): 380-391.
1889 [https://doi380-391.org/10.1061/\(ASCE\)0733-9399\(2002\)128:4\(380\)](https://doi380-391.org/10.1061/(ASCE)0733-9399(2002)128:4(380))
- 1890 [211] Ching J, Chen Y-C. Transitional Markov chain Monte Carlo method for
1891 Bayesian model updating, model class selection, and model averaging. *J. Eng. Mech.*
1892 (2007) 133(7): 816-832. [https://doi.org/10.1061/\(ASCE\)0733-9399\(2007\)133:7\(816\)](https://doi.org/10.1061/(ASCE)0733-9399(2007)133:7(816)).
- 1893 [212] Jensen HA, Vergara C, Papadimitriou C, Millas E. The use of updated robust
1894 reliability measures in stochastic dynamical systems. *Comput. Methods Appl. Mech.*
1895 *Eng.* (2013) 267: 293-317, <https://doi.org/10.1016/j.cma.2013.08.015>.
- 1896 [213] Hadjidoukas PE, Angelikopoulos P, Papadimitriou C, Koumoutsakos P. Π4U:
1897 A high performance computing framework for Bayesian uncertainty quantification of
1898 complex models. *J. Comput. Phys.* (2015) 284: 1-21.
1899 <https://doi.org/10.1016/j.jcp.2014.12.006>.
- 1900 [214] Ching J, Beck JL. Real-time reliability estimation for serviceability limit
1901 states in structures with uncertain dynamic excitation and incomplete output data.
1902 *Probab. Eng. Mech.* (2007) 22(1): 50-62.
1903 <https://doi.org/10.1016/j.probengmech.2006.05.006>.
- 1904 [215] Bansal S, Cheung SH. A new stochastic simulation algorithm for updating
1905 robust reliability of linear structural dynamic systems subjected to future Gaussian
1906 excitations. *Comput. Methods Appl. Mech. Eng.* (2017) 326: 481-504,
1907 <https://doi.org/10.1016/j.cma.2017.07.032>.

1908 [216] Straub D, Papaioannou I, Betz W. Bayesian analysis of rare events. *J.*
1909 *Comput. Phys.*(2016) 314: 538-556. <https://doi.org/10.1016/j.jcp.2016.03.018>.
1910 [217] Straub D, Papaioannou I. Bayesian Updating with Structural Reliability
1911 *Methods. J. Eng. Mech.* (2015) 141(3): 4014134.
1912 [https://doi.org/10.1061/\(ASCE\)EM.1943-7889.0000839](https://doi.org/10.1061/(ASCE)EM.1943-7889.0000839)
1913 [218] Kanjilal O, Papaioannou I, Straub D. Bayesian updating of reliability by
1914 cross entropy-based importance sampling. *Struct. Saf.*(2023) 102: 102325.
1915 <https://doi.org/10.1016/j.strusafe.2023.102325>.
1916 [219] Behmanesh I, Moaveni B, Lombaert G, Papadimitriou C. Hierarchical
1917 Bayesian model updating for structural identification. *Mech. Syst. Signal Process.*
1918 (2015) 64-65: 360–376. <https://doi.org/10.1016/j.ymsp.2015.03.026>
1919 [220] Song M, Behmanesh I, Moaveni B, Papadimitriou C. Accounting for
1920 modeling errors and inherent structural variability through a hierarchical Bayesian
1921 model updating approach: An overview. *Sens.* (2020) 20(14): 3874.
1922 <https://doi.org/10.3390/s20143874>
1923 [221] Jia X, Sedehi O, Papadimitriou C, Katafygiotis LS, Moaveni B. Hierarchical
1924 Bayesian modeling framework for model updating and robust predictions in structural
1925 dynamics using modal features. *Mech. Syst. Signal Process.* (2022) 170: 108784.
1926 <https://doi.org/10.1016/j.ymsp.2021.108784>.
1927 [222] Jia X, Hou W, Papadimitriou C. Hierarchical Bayesian Modeling for
1928 Uncertainty Quantification and Reliability Updating using Data. *ArXiv.*
1929 (2024)2412.20416. <https://doi.org/10.48550/arXiv.2412.20416>.
1930 [223] Faes M G R, Valdebenito M A. Fully decoupled reliability-based design
1931 optimization of structural systems subject to uncertain loads. *Comput. Methods Appl.*
1932 *Mech. Eng.* (2020) 371: 113313. <https://doi.org/10.1016/j.cma.2020.113313>.
1933 [224] Faes M G R, Valdebenito M A. Fully decoupled reliability-based
1934 optimization of linear structures subject to Gaussian dynamic loading considering
1935 discrete design variables. *Mech. Syst. Signal Process.* (2021) 156: 107616.
1936 <https://doi.org/10.1016/j.ymsp.2021.107616>.
1937 [225] Jiang Y B, Zhang X Y, Beer M, et al. An efficient method for
1938 reliability-based design optimization of structures under random excitation by
1939 mapping between reliability and operator norm. *Reliab. Eng. Syst. Saf.* (2024) 245:
1940 109972. <https://doi.org/10.1016/j.ress.2024.109972>.
1941 [226] Weng L L, Acevedo C H, Yang J, et al. An approximate decoupled
1942 reliability-based design optimization method for efficient design exploration of linear
1943 structures under random loads. *Comput. Methods Appl. Mech. Eng.* (2024) 432:
1944 117312. <https://doi.org/10.1016/j.cma.2024.117312>.
1945 [227] Faes M G R, Valdebenito M A, Moens D, et al. Bounding the first
1946 excursion probability of linear structures subjected to imprecise stochastic loading.
1947 *Comput. Struct.* (2020) 239: 106320.
1948 <https://doi.org/10.1016/j.compstruc.2020.106320>.
1949 [228] Faes M G R, Valdebenito M A, Moens D, et al. Operator norm theory as an
1950 efficient tool to propagate hybrid uncertainties and calculate imprecise probabilities.
1951 *Mech. Syst. Signal Process.* (2021) 152: 107482.
1952 <https://doi.org/10.1016/j.ymsp.2020.107482>.
1953 [229] Ni P, Jerez D J, Fragkoulis V C, et al. Operator norm-based statistical
1954 linearization to bound the first excursion probability of nonlinear structures subjected
1955 to imprecise stochastic loading. *ASCE-ASME J. Risk Uncert. Eng. Syst., Part A: Civ.*
1956 *Eng.* (2022) 8(1): 04021086. <https://doi.org/10.1061/AJRUA6.0001217>.
1957 [230] Jerez D J, Fragkoulis V C, Ni P, et al. Operator norm-based determination
1958 of failure probability of nonlinear oscillators with fractional derivative elements
1959 subject to imprecise stationary Gaussian loads. *Mech. Syst. Signal Process.* (2024)
1960 208: 111043. <https://doi.org/10.1016/j.ymsp.2023.111043>.